

15 September 2017

HUAPAI TRIANGLE SUB PRECINCT A, STAGE 1C

GEOTECHNICAL COMPLETION REPORT

Cabra Developments Limited

Ref: AKL2016_0634AE Rev: 1

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1. INTRODUCTION

In accordance with our instructions, this Geotechnical Completion Report has been prepared for Cabra Developments Limited as part of the documentation to be submitted to Auckland Council following earthworks to form Stage 1C of the Huapai Triangle Sub Precinct A development. Construction of this residential subdivision has been undertaken in accordance with the Auckland Council Resource Consent number SUB-65117 and Engineering Approval letter dated 23 February 2017. Specific structures constructed during the civil works to create the subdivision include timber pole retaining walls and segmental block retaining walls, and two timber boardwalks across the stormwater utility reserve.

This report contains our Suitability Statement, specific comments related to items raised in the Resource Consent, relevant test data and the Cato Bolam Limited as-built plan set as provided in Appendix B.

This report covers the construction period December 2016 to September 2017 and is intended to be used for certification purposes for new lots (listed below) created from Lot 119 DP 508664 as follows:

- 38 new residential lots numbered 93 to 130, with lots 93, 97 and 101 being superlots;
- 1 new road numbered lot 1006 and named Jane Maree Road;
- 1 new stormwater utility reserve numbered lot 1000;
- 1 new local purpose accessway numbered lot 1111; and,
- 1 new right of way numbered lot 501.

This stage of the Huapai Triangle Sub Precinct A is located off Vinistra Road. As can be seen from the as-built plans, 15 of the lots have been affected by filling as part of the earthworks operations to a maximum depth of approximately 2.5 metres.

2. PROJECT BACKGROUND

The geotechnical investigations and design were undertaken by Coffey Geotechnics (NZ) Limited and CMW Geosciences (NZ) Limited as presented in the following reports:

- Geotechnical Constraints Report prepared by Coffey Geotechnics, referenced GENZAUCK16252AA, and dated 12 March 2014.
- Geotechnical Investigation Report prepared by CMW Geosciences, referenced 2015_1029AB Rev. 0, and dated 24 November 2014.
- Retaining Wall Design Report prepared by CMW Geosciences, referenced AKL2016_0634AV Rev. 0, and dated 20 January 2017.

3. DESCRIPTION OF EARTHWORKS

Earthworks operations for the development began in early December 2016 with the installation of silt fences and other environmental controls. The temporary silt ponds for this stage were located on the neighbouring stages. Cut/Fill for this stage was conducted in conjunction with the other stages of this development and with neighbouring developments, all of which were under the observation of CMW Geosciences. The majority of this stage is in areas of cut, with a small area of filling required to form the western lots and the stormwater 'greenfinger' which borders the stage to the west and north.

By January 2017 the bulk earthworks for the stage were mostly complete. An area of organically stained material was observed in the south-eastern corner of the site, affecting road 12 and some of the surrounding lots. This material was undercut as part of the road subgrade formation.

Civil works including road construction began in January 2017 and continued until stage completion in July 2017.

Construction of the timber pole and segmental block retaining walls began in February 2017. The timber pole retaining walls were completed by March 2017 and the segmental block retaining walls in June 2017.

The two timber boardwalks across the stormwater utility reserve and the associated gabion structures were constructed in August 2017.

4. GEOTECHNICAL QUALITY CONTROL

4.1. Site Observations

During the earthworks site visits were typically undertaken several times each week to assess compliance with NZS 4431 and specific design recommendations and specifications.

Site visits were carried out to observe and confirm compliance relating to:

- Adequate topsoil stripping;
- Fill areas prior to the placement of fill materials to ascertain that all mullock and soft inorganic subsoils had been removed;
- Excavation and backfilling of sewer and stormwater trenches;
- Retaining wall pile excavations;
- Construction of cantilever pole retaining walls including ground conditions, pile size, spacing and depth;
- Construction of keystone walls including ground conditions, block placement, geogrid placement and hardfill backfill;
- Placement and compaction of engineered fills; and,
- Installation of subsoil drainage.

4.2. Compaction Control

Compaction of engineered earth fills was controlled by undrained shear strength measured by hand held shear vane calibrated using the NZGS 2001 method and by air voids as defined by NZS4402.

General Fills

The criteria for undrained shear strength were a minimum single value of 110 kPa and minimum average of any 10 consecutive tests of 140 kPa.

The criteria for air voids were a maximum single value of 12% and maximum average of any 10 consecutive tests of 10%.

Vane shear strength, water content and in situ density tests were carried out on all areas of the engineered filling to at least the frequency recommended by NZS 4431.

These tests showed on multiple occasions that the required compaction standards were not being achieved due to the fill materials being too wet, and to the best of our knowledge the failing areas of fill were re-worked as necessary. Subsequent testing confirmed compliance with the specification.

5. EVALUATION OF COMPLETED EARTHWORKS

5.1. Natural Hazards

The appended as-built drawings depict the extents of zones that contain limitations intended to ensure that future building and/ or earthworks on the lots is undertaken in a manner that does not lead to buildings being subject to any of the natural hazards described in section 106 of the Resource Management Act, i.e. erosion, falling debris, subsidence, slippage, and inundation. Consideration of the inundation hazard was outside the scope of CMW's brief and has been assessed by others. The applied zones include:

- **Specific Design Zones (retaining)** - intended to protect the retaining walls from overloading at the crest or undermining at the toe that could lead to instability;
- **Specific Design Zones (slope)** – intended to protect building development from long term creep effects on or adjacent to steep slopes and to protect the slopes from inappropriate loading or undermining.

Full descriptions of the restrictions associated with these zones are presented in the Suitability Statement (Appendix A) and their locations are depicted on the appended Cato Bolam Limited As-built Plans. Additional information is also provided in some of the following sections.

5.2. Natural Soils Geotechnical Assessment

Portions of lots within this stage of the subdivision are formed partially within natural soils, which are predominantly of alluvial origin.

During construction and earthworks, lenses of firm to stiff organically stained clay soils were observed and were undercut and replaced with engineered fill.

Hand auger boreholes conducted in the centre of each lot as part of the post earthworks investigations did not encounter further lenses of organically stained soils, but observed relatively low strength natural subsoils.

While not expected, some discrete lenses of organic stained clay soils may still exist within the natural soils beneath lot areas. We do not consider that liquefaction and/or settlement due to discrete lenses of organically stained clay soils are a significant geotechnical risk for development or future residential dwellings built in general accordance with NZS3604.

5.3. Land Stability and Erosion Control

Building and landscape designers must ensure that all runoff from solid surfaces is directed into the stormwater system. It is also important that care is paid to the disposal of stormwater during construction so that concentrated discharges (e.g. from unconnected spouting) are not directed towards steep ground.

Depths of mulch and topsoil applied to sloping areas should be limited to less than 150mm to minimise the risks of saturation leading to localised slumping on batter face. Wherever practical on such land, existing vegetation and grass cover should be well maintained. Any vegetation cleared beyond the immediate area of building platforms for temporary construction purposes should be replanted or replaced as soon as possible. The roots of an established vegetation cover can serve to bind the surface soils while the foliage can reduce rain infiltration and soil saturation, resulting in better resistance to erosion and shallow slumping.

5.4. Retaining Walls

Cantilever pole retaining walls and segmental block walls have been constructed in the locations shown on the appended Cato Bolam Limited As-built Plan. These walls reach a maximum height of

approximately 2.0 metres and were designed by CMW Geosciences, approved under BCO10090856, and the construction was observed by this consultancy. Copies of the Producer Statements - Construction Review are provided in Appendix E.

Descriptions of the building and earthworks restrictions within the vicinity of these walls (Specific Design Zones – retaining) are contained in the Suitability Statement in Appendix A. Lots containing these zones are 108 to 130 inclusive.

5.5. Fill Induced Settlement

On the basis of the relatively minor magnitude of fill depths on this site, together with the elapsed time since it was placed, we consider that remaining post-construction settlements will be within code limits.

5.6. Service Line Trenches

As part of the civil works, sanitary sewer and stormwater services were trenched throughout the development as shown on the appended Cato Bolam Limited Stormwater and Sanitary Sewer As-built Plans.

As is normal on all subdivisions, building developments involving foundations within a 45 degree zone of influence from pipe inverts will require engineering input. The Auckland Council drawing referenced SW22 provided in Appendix B extracted from Chapter 4 of the Auckland Council Code of Practice for Land development and Subdivision depicts their requirements for stormwater pipes. Details for water and wastewater pipes are available in the Watercare COP1 - General Requirements and Procedures. The majority of lots are known to have service trenches within the lots as shown on the appended stormwater and wastewater as-built plans. The resulting restrictions are presented in the Suitability Statement below.

5.7. Stormwater Detention Device

A stormwater detention device has been constructed as part of this stage of the subdivision. This device is formed as a series of vegetated swales connected by stormwater pipes and rock armouring surrounding the inlets and outlets of the pipes, with the device becoming broader within drainage reserve lot 1000.

The device was formed mostly in fill within the southern portion of the swale and within cut natural ground within the northern portion of the swale and in broader areas of lot 1000. Ground conditions were inspected by CMW during construction of the swale to confirm the soils comprised plastic clays for the lining of the device.

Permeability of the device liner was confirmed via triaxial cell permeability tests of the in-situ soils, with a hydraulic conductivity (k) in the order of 10^{-10} m/s.

On the basis of the construction observations undertaken during the subdivision formation we are satisfied that the stormwater detention device has been constructed in accordance with the design and is suitable for its intended use.

5.8. Road Subgrades

Penetration resistance testing was carried out on the road subgrades during construction and the results of this testing were forwarded to Cato Bolam Limited for pavement remedial design. Where soft ground with low equivalent CBR values was identified it was generally undercut and replaced with engineered clay fill or hardfill. All road subgrade areas were subsequently lime stabilised to achieve appropriate CBR values.

Benkelman Beam testing of the subgrade and base course was carried out by Road Test Limited on each road and those results were also forwarded to Cato Bolam Limited.

5.9. Design of Shallow Foundations

5.9.1. Bearing Capacity

Once bulk earthworks and top-soiling of the building platforms had been completed, our staff drilled hand auger boreholes on platforms in natural ground to determine representative finished ground conditions and hence evaluate likely foundation options for future building development. Our assessments of bearing capacity for the design of shallow foundations on each building platform are contained in the appended Suitability Statement.

At current subgrade levels lots 93 to 105, 111 to 116, and 121 to 130 inclusive have been assessed as having a geotechnical ultimate bearing capacity of 300 kPa within the influence of conventional shallow residential building foundation loads. However on account of the presence of lower strength natural sub soils, a geotechnical ultimate bearing capacity of 240 kPa has been assessed for lots 106 to 110 and 117 to 120 inclusive.

If higher geotechnical ultimate bearing capacities are required, further specific site investigation and design of foundations should be carried out prior to Building Consent application.

5.9.2. Foundation Settlements

At the bearing pressures specified above and subject to the design requirements for soil expansiveness provided below, differential settlement of shallow foundations for buildings designed in accordance with NZS 3604 (including the 600mm subfloor fill depth limit) should be within code limits.

5.9.3. Soil Expansiveness Classification

3 sets of soil tests were carried out on samples taken from likely foundation level on lots within this stage of the development.

Testing was carried out in accordance with NZS 4402, "Methods of Testing Soils for Civil Engineering Purposes" test 2.2 and 2.6 and were used in conjunction with visual-tactile assessment of the site soils to determine expansive site Classes as defined in AS 2870, "Residential Slabs and Footings – Construction". All test results are appended.

On this basis we have assessed the AS 2870 Site Class for all lots these stages of the development to be H1 (high). Details of foundation options for this Class are contained in the appended Suitability Statement.

In recent years in Auckland, there have been examples of concrete floors and/ or foundations that have been poured on dry, desiccated subgrades in summer months on expansive soils and have undergone heaving and cracking once the soil moisture contents have returned to higher levels. Foundation contractors need to be made aware of this issue and the need to maintain appropriate moisture contents in the footings and building platform subgrade between the time of excavation and the pouring of concrete.

Remedial actions that may be appropriate include platform protection with a hard fill layer, pouring of a blinding layer of concrete in footing bases and soaking of the building platform with sprinklers for an extended period.

Home owners need to be aware that the planting of high water demand plants where their roots may extend close to footings can also cause settlement damage.

5.10. Topsoil Depths

Topsoil depths have been checked by the drilling of a borehole in the approximate centre of the building platform on each lot. The results are considered indicative for each lot, but may be subject to variations. Topsoil depths are between 150 and 300mm on this stage of the development.

Site specific findings are contained in the appended Suitability Statement Summary (Appendix A). However, it is possible that further levelling works have been undertaken since our investigations and accordingly, we strongly recommend that lot purchasers complete their own checks of topsoil depths.

6. CLOSURE

The appended Statement of Professional Opinion is provided to the Auckland Council and Cabra Developments Limited for their purposes alone on the express condition that it will not be relied upon by any other person. It is important that prospective purchasers satisfy themselves as to any specific conditions pertaining to their particular land interest.

Although regular site visits have been undertaken for observation, for providing guidance and instruction and for testing purposes, the geotechnical services scope did not include full time site presence. To this end, our appended Suitability Statement also relies on the Contractors' work practices and assumes that when we have not been present to observe the work, it has been completed to high standards and in accordance with the drawings, instructions and consent conditions provided to them.

Similarly it assumes that all as-built information and other details provided to the Client and/or CMW by other members of the project team are accurate and correct in all respects.

For and on behalf of CMW Geosciences

Prepared by:



Jack Mynett - Johnson

Engineering Geologist

Reviewed and Approved by:



Richard Knowles

Principal Geotechnical Engineer, CPEng

Appendix A

Statement of Professional Opinion as to the Suitability of Land for Building Development

STATEMENT OF PROFESSIONAL OPINION AS TO THE SUITABILITY OF LAND FOR BUILDING DEVELOPMENT

I, Richard Knowles, of CMW Geosciences, Auckland, hereby confirm that:

1. As a Chartered Professional Engineer experienced in the field of geotechnical engineering, I am a Geo-professional as defined in section 1.2.2 of NZS 4404 and was retained by the Developer as the Geotechnical Engineer on Stage 1C of the Huapai Triangle Sub Precinct A development.
2. The extent of preliminary investigations carried out to date are described in the CMW Geosciences (NZ) Limited Geotechnical Investigation Report referenced 2015_1029AB Rev. 0, dated 24 November 2014. The conclusions and recommendations of those documents have been re-evaluated in the preparation of this report. The results of all tests carried out are also appended.
3. In my professional opinion, not to be construed as a guarantee, I consider that:
 - (a) The earth fills shown on the appended Cato Bolam Cut/Fill As-built Plans have been placed in compliance with NZS 4431, the Legacy Rodney District Council Plans and related documents.
 - (b) The completed earthworks give due regard to land slope and foundation stability considerations on the building platform areas, but as shown on the appended building restriction zones plans, areas on all lots have gradients steeper than 1(v) in 4 (h) or are adjacent to land having such gradients. Accordingly, restrictions incorporating Specific Design Zones (Slope) have been applied as depicted on the as-built plans as follows:
 - **Specific Design Zone (Slope) areas** have been applied on Lots 93 to 95 and 102 inclusive. No building construction and no earthworks (i.e. cut or fills of any depth) should take place within the designated **Specific Design Zone (Slope) areas** unless endorsed by a Chartered Professional Engineer experienced in geomechanics and familiar with the contents of this report. The endorsement will need to consider the implications of the proposals on both global stability conditions and soil creep on the buildings, the interaction with service pipes and associated trench backfills, control of surface water, construction sequencing, timing and temporary support requirements construction of all earthworks, foundations and retaining walls and if necessary, comment on what aspects require engineering inspections and certification.

This limitation also applies to long term landscaping works, including any proposed minor cuts either on or near batter toes to be retained by new landscaping walls that might not normally require engineering, and to landscaping fills on or immediately above the batter slopes.

- (c) **Specific Design Zone (Retaining Walls) areas** have been applied on Lots 108 to 130 inclusive for the protection of the function of the retaining walls. The retaining walls on this stage of the development were designed for a maximum of 12kPa surcharge load and 5° toe slope. No building construction and no earthworks (i.e. cut or fills) should take place that exceed these design limits on the walls unless endorsed by a Chartered Professional Engineer experienced in geomechanics and familiar with the contents of this report.
- (d) A geotechnical ultimate bearing capacity of 300 kPa may be assumed for shallow foundation design on the building platforms of Lots 93 to 105, 111 to 116, and 121 to 130 inclusive.

Due to the presence of softer natural subsoils on the building platforms of Lots 106 to 110 and 117 to 120 inclusive a geotechnical ultimate bearing capacity of 240 kPa may be assumed for shallow foundation design on these lots.

If for any reason higher geotechnical bearing capacities are required, further specific site investigation and design of foundations should be carried out prior to Building Consent application.

- (e) The expansive site Class for all lots has been assessed as AS2870 Class H1 (High). We recommend that building designers note on the Building Consent drawings the need to maintain appropriate moisture levels across building subgrades and in footing excavations (as described in Section 5.9.3 of the Geotechnical Completion Report) for reference by foundation contractors.
- (f) The backfilling and compaction of the storm water and sanitary sewer trenches on this subdivision has been carried out to appropriate standards having regard for the prevailing ground conditions and associated compaction induced pipe loadings.

However, no building development should take place within the 45 degree zone of influence of drain inverts unless endorsed by specific design and by construction inspections undertaken by a Chartered Professional Engineer experienced in geomechanics to ensure that lateral stability and differential settlement issues are addressed and that building loads are transferred beyond the influence of the pipe and trench backfill. A copy of drawing SW22 extracted from Chapter 4 of the Auckland Council Code of Practice for Land development and Subdivision this document is provided in Appendix B for clarification. Details for water and wastewater pipes are available in the Watercare COP1 - General Requirements and Procedures.

- (g) Subject to the geotechnical limitations, restrictions and recommendations contained in clauses 3(b), 3(c), 3(d), 3(e), and 3(f) above:
 - (i) The filled and natural ground is generally suitable for residential buildings constructed in accordance with NZS 3604 and the requirements of AS2870 for the appropriate expansive soil class.
 - (ii) Where shallow foundations are appropriate, design may be carried out in accordance with AS 2870 (Class H1) or alternately, a specific foundation and structural design may be undertaken by a Chartered Professional Engineer.
- 4. Road subgrades have been formed with appropriate regard for slope stability and settlement risks.
- 5. The permanent storm water treatment device in Lot 1000 has been formed to standards appropriate for its intended use.

The following table summarises the conditions on each of each residential lots.

For and on behalf of
CMW Geosciences (NZ) Limited



Richard Knowles

Principal Geotechnical Engineer, CPEng

GCR Summary Table

Condition	Specific Design Zone (slope)	Specific Design Zone (retaining)	Geotechnical Ultimate Bearing Capacity (kPa)	AS2870 Expansive Class	Service Lines Restrictions	Indicative Topsoil Depth (mm)
GCR SOPO Clause	3(b)	3(c)	3(d)	3(e)	3(f)	
Lot number						
93	●		300	H1	●	300
94	●		300	H1	●	250
95	●		300	H1	●	300
96			300	H1	●	250
97			300	H1	●	200
98			300	H1	●	300
99			300	H1	●	300
100			300	H1	●	150
101			300	H1	●	300
102	●		300	H1	●	250
103			300	H1	●	250
104			300	H1	●	300
105			300	H1	●	300
106			240	H1	●	300
107			240	H1	●	250
108		●	240	H1	●	300
109		●	240	H1	●	300
110		●	240	H1	●	300
111		●	300	H1	●	250

Condition	Specific Design Zone (slope)	Specific Design Zone (retaining)	Geotechnical Ultimate Bearing Capacity (kPa)	AS2870 Expansive Class	Service Lines Restrictions	Indicative Topsoil Depth (mm)
112	●	300	H1	●	300	
113	●	300	H1	●	300	
114	●	300	H1		300	
115	●	300	H1		300	
116	●	300	H1		250	
117	●	240	H1	●	300	
118	●	240	H1	●	150	
119	●	240	H1	●	300	
120	●	240	H1	●	300	
121	●	300	H1	●	300	
122	●	300	H1	●	300	
123	●	300	H1	●	300	
124	●	300	H1	●	300	
125	●	300	H1	●	300	
126	●	300	H1	●	300	
127	●	300	H1	●	300	
128	●	300	H1	●	300	
129	●	300	H1	●	300	
130	●	300	H1	●	300	

Appendix B

Drawings

Title	Reference No.	Date	Revision
Cato Bolam Consultants Final Contours and Retaining Walls As-built Plans	34745 E600-603	July 2017	-
Cato Bolam Consultants Cut Fill As-built Plans	34745 E604-607	July 2017	-
Cato Bolam Consultants Undercut As-built Plans	34745 E608-610	July 2017	-
Cato Bolam Consultants Roading As-built Plans	34745 E611-613	July 2017	-
Cato Bolam Consultants Wastewater Reticulation As-built Plans	34745 E614-616	July 2017	-
Cato Bolam Consultants Stormwater As-built Plans	34745 E617-621	July 2017	-
Cato Bolam Consultants Water Reticulation As-built Plans	34745 E622-624	July 2017	-
Cato Bolam Consultants Retaining Wall Specific Design Zone Plan As-built Plans	34745 E625	July 2017	-
Cato Bolam Consultants Drainage Zone of Influence As-built Plans	34745 E626	July 2017	-
Cato Bolam Consultants Slope Specific Design Plan	34745 E627	July 2017	-
Cato Bolam Consultants Telecom As-built Plans	34745 E629-631	July 2017	-
Auckland Council Stormwater Pipe and Manhole Construction Clearance Requirements	ACSD SW22	Sept 2013	1

Cabra Developments Ltd

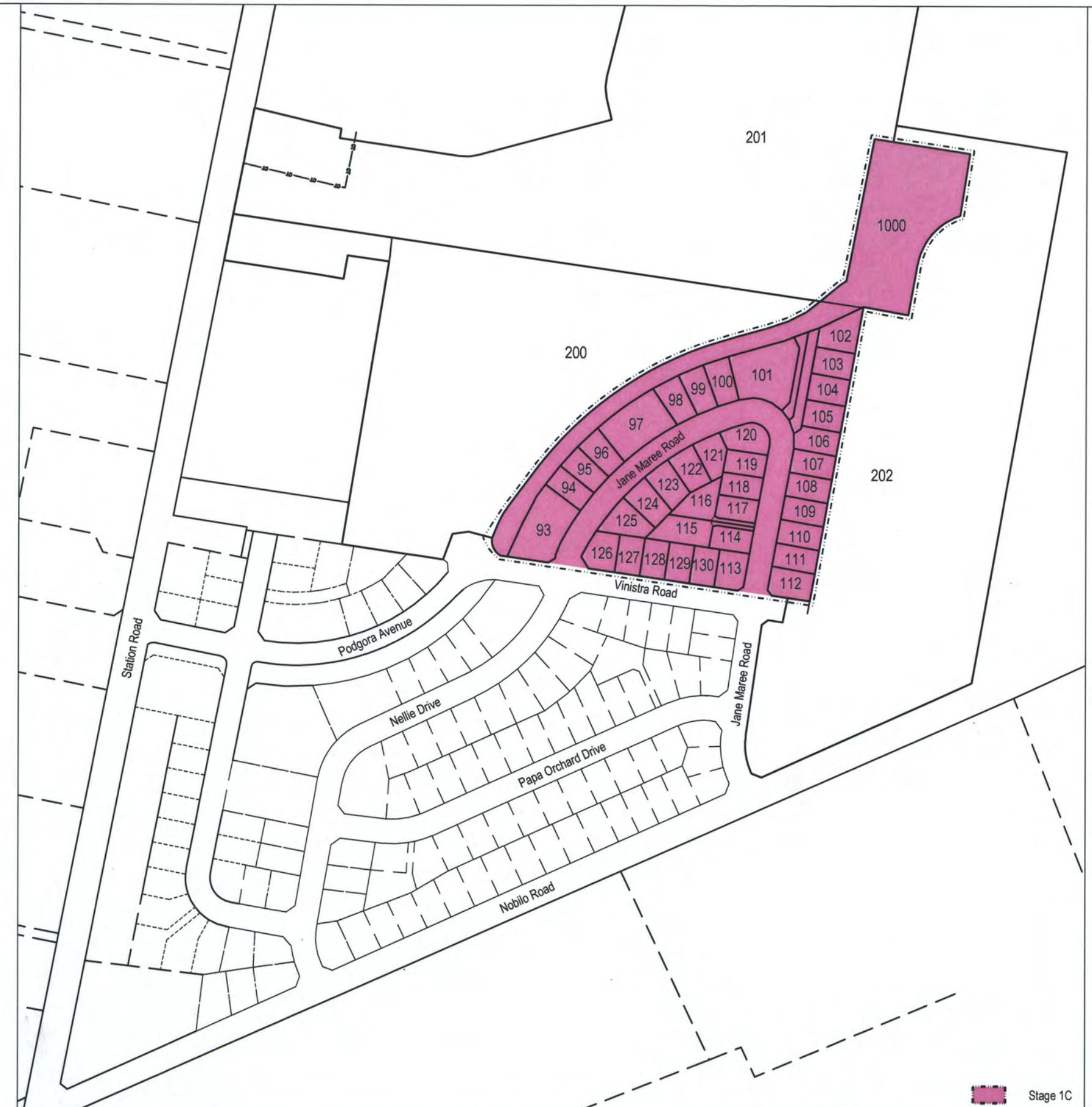
Huapai Triangle

Sub Precinct A

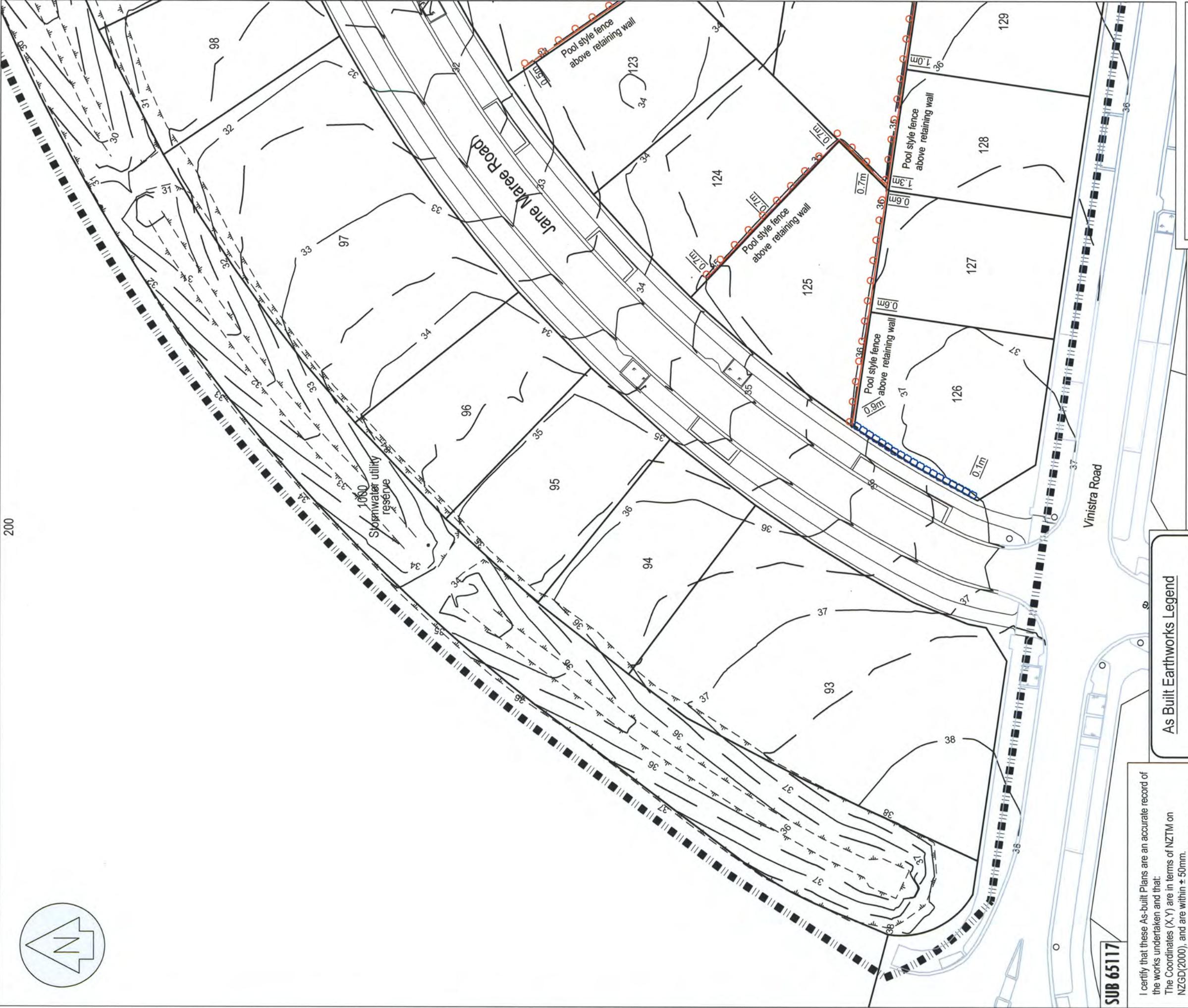
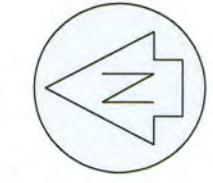
Stage 1C Asbuilt Plans

As Built Plan Set

E00	Cover Sheet
E600	Final Contours & Retaining Walls - Sheet 1 of 4
E601	Final Contours & Retaining Walls - Sheet 2 of 4
E602	Final Contours & Retaining Walls - Sheet 3 of 4
E603	Final Contours & Retaining Walls - Sheet 4 of 4
E604	Cut Fill - Sheet 1 of 4
E605	Cut Fill - Sheet 2 of 4
E606	Cut Fill - Sheet 3 of 4
E607	Cut Fill - Sheet 4 of 4
E608	Lots 118-120, 108, 109 & Road undercut and backfilled with certified clay - Sheet 1 of 3
E609	Lots 118-120, 108, 109 & Road undercut and backfilled with certified clay - Sheet 2 of 3
E610	Lots 118-120, 108, 109 & Road undercut and backfilled with certified clay - Sheet 3 of 3
E611	Road - Sheet 1 of 3
E612	Road - Sheet 2 of 3
E613	Road - Sheet 3 of 3
E614	Wastewater Reticulation - Sheet 1 of 3
E615	Wastewater Reticulation - Sheet 2 of 3
E616	Wastewater Reticulation - Sheet 3 of 3
E617	Stormwater Reticulation - Sheet 1 of 4
E618	Stormwater Reticulation - Sheet 2 of 4
E619	Stormwater Reticulation - Sheet 3 of 4
E620	Stormwater Reticulation - Sheet 4 of 4
E621	Stormwater Attenuation Area A - Sheet 1 of 1
E622	Water Reticulation - Sheet 1 of 3
E623	Water Reticulation - Sheet 2 of 3
E624	Water Reticulation - Sheet 3 of 3
E625	Retaining Wall Specific Design Plan - Sheet 1
E626	Drainage Zone of Influence Plan - Sheet 1
E627	Telecom Network



LOCATION DIAGRAM



SUB 65117

I certify that these As-built Plans are an accurate record of the works undertaken and that the coordinates (X, Y) are in terms of NZTM on NZGD2000, and are within ± 50 mm. The levels (Z) are in terms of the Auckland 1946 (MSL) LINZ datum (DOSL datum), and are within ± 25 mm.

Signed:
.....
Licensed Cadastral Surveyor
Date:
11/07/2017
Name: Michelle Bain
Phone: 09 427 0072
Email: catobolam@catobolam.co.nz

As Built Earthworks Legend

- 35 — As built Contour (1m interval)
- 35 — As built Contour (0.5m interval)
- As built Timber Pole Retaining Wall (Poles not shown to scale)
- As built Concrete Keystone Retaining Wall
- Retaining Wall Height 1.0m
- ■ ■ ■ ■ Extent of SUB-65117 Stage 1C As Built works

NOTES

1. Contours are finished ground levels as surveyed at 21/07/2017. Contour interval of 0.5m. Levels are in terms of LINZ Datum 1946.
2. 3. Coordinates are in terms of NZTM.
4. All retaining walls over 1.0m high have a pool style fence above the wall.
5. Retaining walls approved and constructed under ABA BC 10090856

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This plan and accompanying report(s) have been prepared for the purpose of obtaining a resource consent only and for no other purpose. Use of this plan and/or the information on it for any other purpose is at the user's risk.

CATO BOLAM CONSULTANTS
PLANNERS ENGINEERS
SURVEYORS

CATO BOLAM CONSULTANTS LTD
19 Tumaniki Avenue
PO Box 157
Orewa 0946
phone 09-427 0072
fax 09-426 7331
email catobolam@catobolam.co.nz

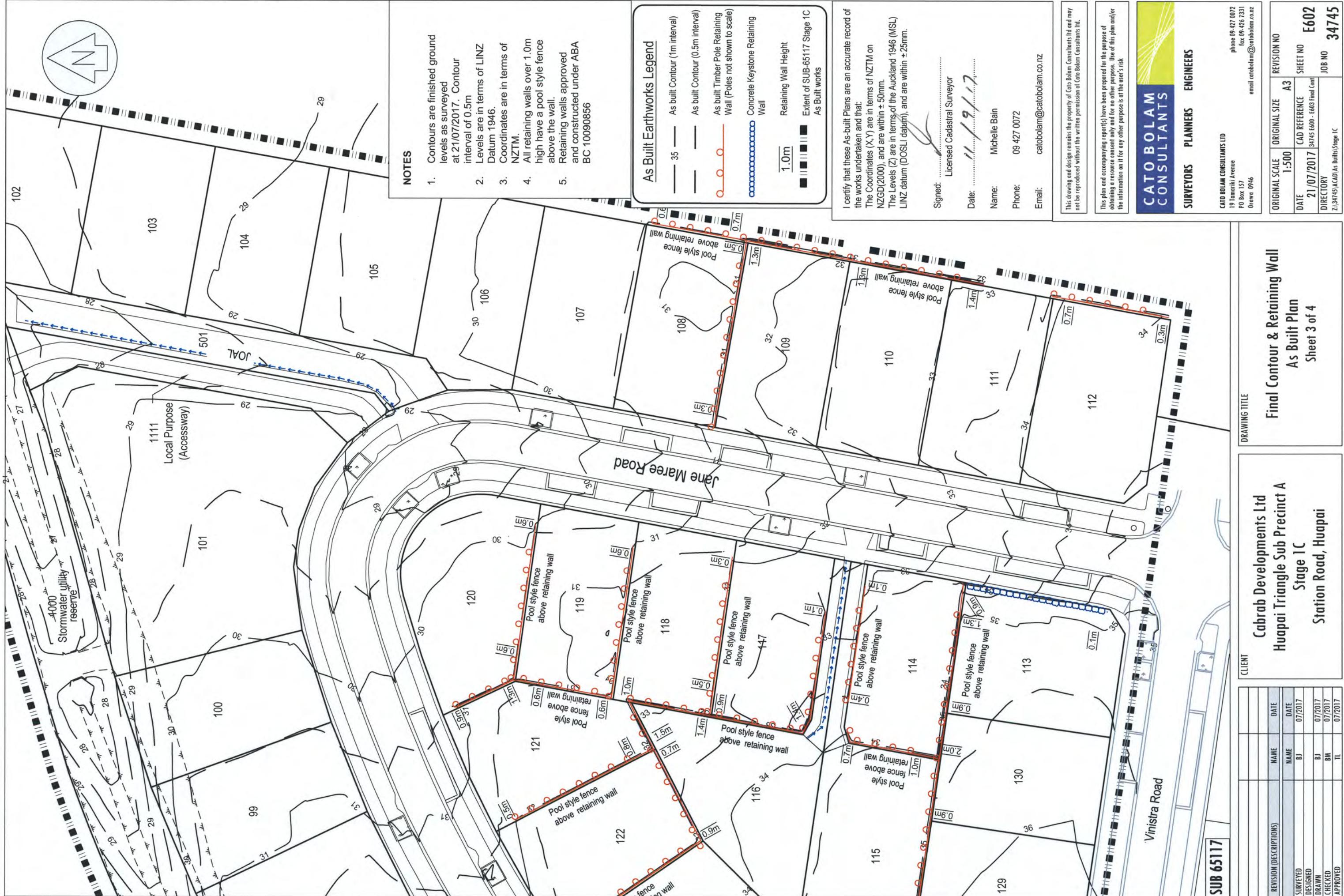
ORIGINAL SCALE 1:500 CAD REFERENCE A3 SHEET NO E601
DATE 21/07/2017 DIRECTORY 34745 E601 Final Contol
JOB NO 34745

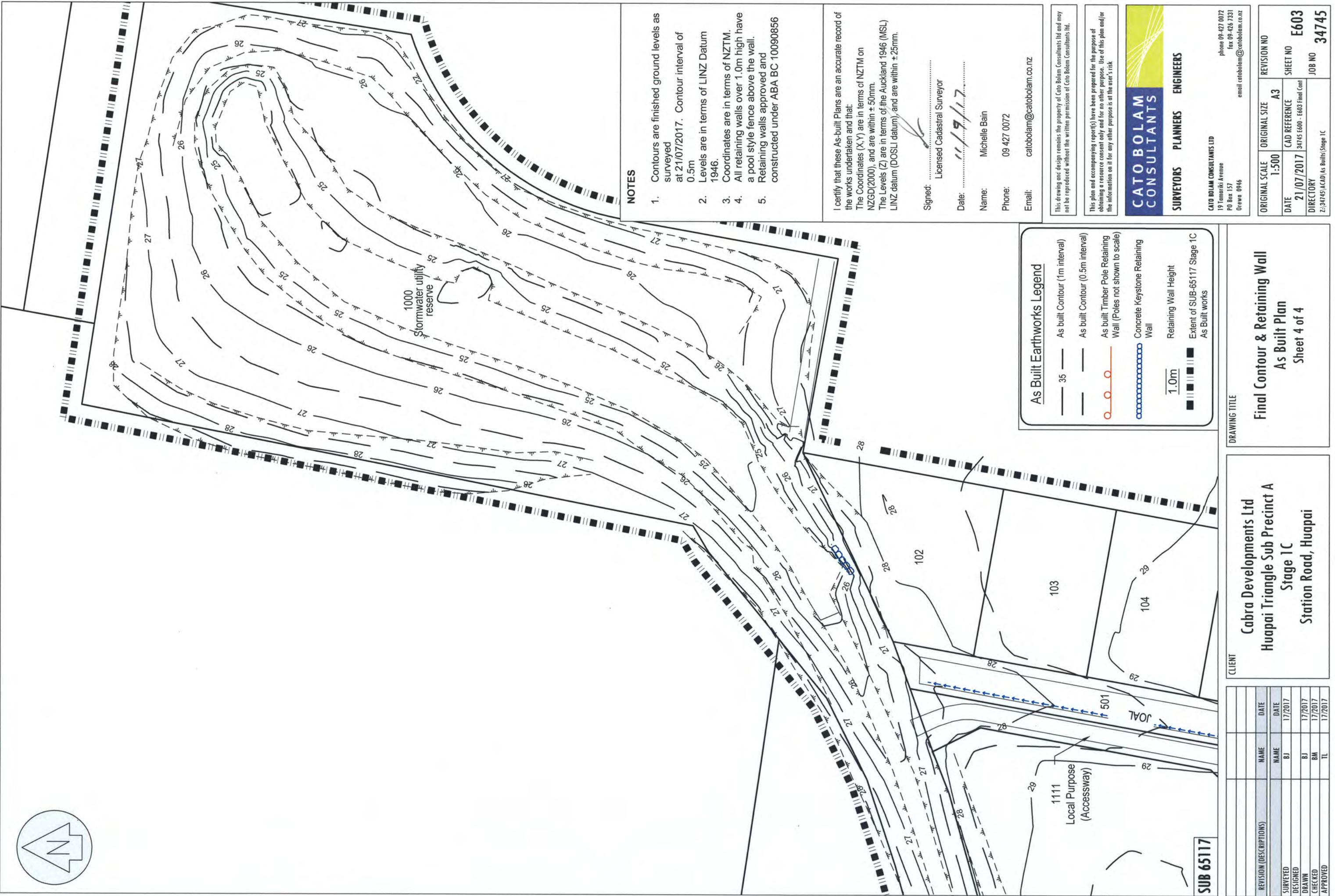
Final Contour & Retaining Wall
As Built Plan
Sheet 2 of 4

DRAWING TITLE

CLIENT
Cabra Developments Ltd
Huapai Triangle Sub Precinct A
Stage 1C
Station Road, Huapai

REVISION (DESCRIPTIONS)	NAME	DATE	ORIGINAL SIZE	REVISION NO
SURVEYED	BJ	07/2017	CAD REFERENCE	A3
DESIGNED	BJ	07/2017	DATE	21/07/2017
DRAWN	BM	07/2017	SHEET NO	E601
CHECKED	TL	07/2017	DIRECTORY	34745 E601 Final Contol
APPROVED			JOB NO	34745







See sheet E607 for details

201

Refer to sheet E605 for details

200

20

Refer to sheet E606
for details

I certify that these As-built Plans are an accurate record of the works undertaken and that:

The Coordinates (X,Y) are in terms of NZTM or NZGD(2000), and are within $\pm 50\text{mm}$.

The Levels (Z) are in terms of the Auckland 1946 (MSL) LINZ datum (DOSLI datum) and are within $\pm 25\text{mm}$.

LINE datum (DOORI datum), and are within $\pm 20\text{m}$.

Signed:
Licensed Cadastral Surveyor

Date: 11/9/17

Name: Michelle Ba

Phone: 09 427 007

Email: catoholam@catoholam.co.m

Map showing property boundaries and road names. Key features include:

- Roads:** Nellie Drive, Vinstria Road, Jane Maree Road.
- Properties:**
 - Lot 205 DP 507583
 - Lot 93 DP 507583
 - Lot 107 DP 508664
 - Lot 106 DP 508664
 - Lot 105 DP 508664
 - Lot 104 DP 508664
 - Lot 103 DP 508664
 - Lot 102 DP 508664
 - Lot 99 DP 507583
 - Lot 100 DP 507583
 - Lot 101 DP 507583
 - Lot 98 DP 507583
 - Lot 96 DP 507583
 - Lot 97 DP 507583
 - Lot 95 DP 507583
 - Lot 94 DP 507583
 - Lot 92 DP 507583
 - Lot 91 DP 507583
 - Lot 90 DP 507583
 - Lot 89 DP 507583
- Area:** SUB 65117

NOTES GENERAL

GENERAL

1. Levels are in terms of LINZ Datum 1946.
2. Coordinates are in terms of NZTM.
3. All infrastructure is public unless otherwise shown.
4. Cut/fill contours are from the original ground level, to finished ground level as surveyed on 21/07/2017. Excludes undercuts which are shown on plans F608 - F610.

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Cut to Fill Legend

REVISION (DESCRIPTIONS)	NAME	DATE
SURVEYED	BJ	07/2017
DESIGNED		
DRAWN	BJ	07/2017
CHECKED	BM	07/2017
APPROVED	TI	07/2017

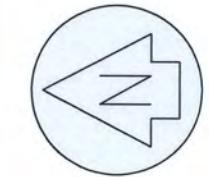
CATO BOLAM
CONSULTANTS

SURVEYORS **PLANNERS** **ENGINEERS**

CLIENT Cabra Developments Ltd
Huapai Triangle Sub Precinct A
Stage 1C
Station Road, Huapai

DRAWING TITLE
**Cut Fill
As Built Plan
Sheet 1 of 4**

ORIGINAL SCALE 1:1500	ORIGINAL SIZE A3	REVISION NO
DATE 21/07/17	CAD REFERENCE 34745 E600 - E603	SHEET NO E604
DIRECTORY Z:\34745\ACAD\As Built\Stage 1C		JOB NO 34745



I certify that these As-built Plans are an accurate record of the works undertaken and that:
The Coordinates (X,Y) are in terms of NZTM on NZGD2000, and are within ± 50 mm.
The Levels (Z) are in terms of the Auckland 1946 (MSL) LINZ datum (DOSL1 datum), and are within ± 25 mm.

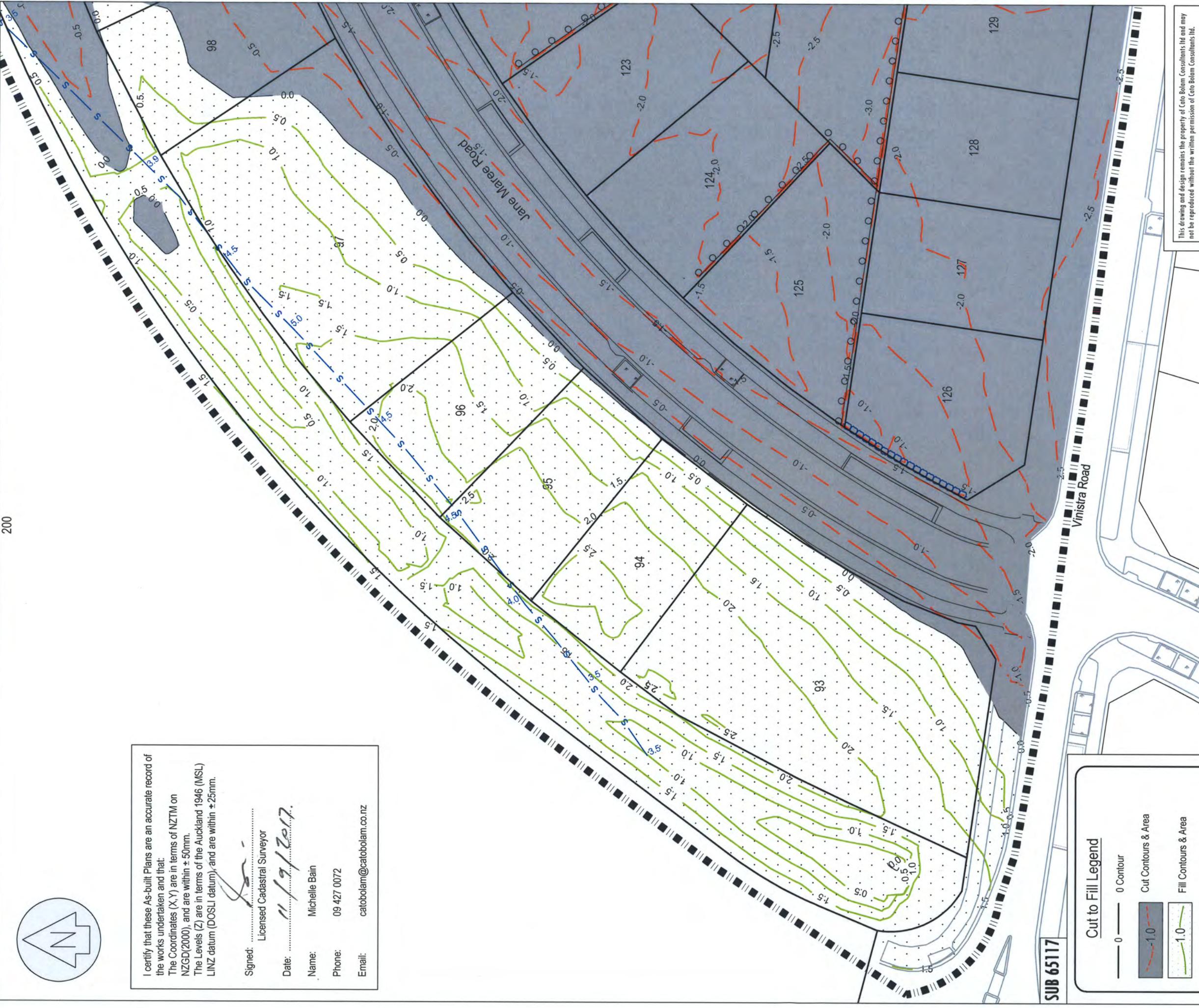
Signed: Licensed Cadastral Surveyor

Date: 11/07/2017.

Name: Michelle Bain

Phone: 09 427 0072

Email: catobolam@catobolam.co.nz



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CATO BOLAM CONSULTANTS
ENGINEERS PLANNERS SURVEYORS

CATO BOLAM CONSULTANTS LTD
19 Tannik Avenue
PO Box 157
Drews 0946

phone 09 427 0072
fax 09 426 7331
email catobolam.co.nz

ORIGINAL SCALE	ORIGINAL SIZE	REVISION NO
1:500	A3	E605
DATE	CAD REFERENCE	SHEET NO
21/07/2017	34745_E604 - E607 Cut Fill	
DIRECTORY		JOB NO
Z:\34745\ACD\As Built\Stage 1C		34745

Cut Fill As Built Plan Sheet 2 of 4

93 507583 107 DP 508664

DRAWING TITLE

NOTES GENERAL

1. Levels are in terms of LINZ Datum 1946.
2. Coordinates are in terms of NZTM.
3. All infrastructure is public unless otherwise shown.
4. Cut/fill contours are from the original ground level to finished ground level as surveyed on 21/07/2017. Excludes undercutts which are shown on plans E608 - E610

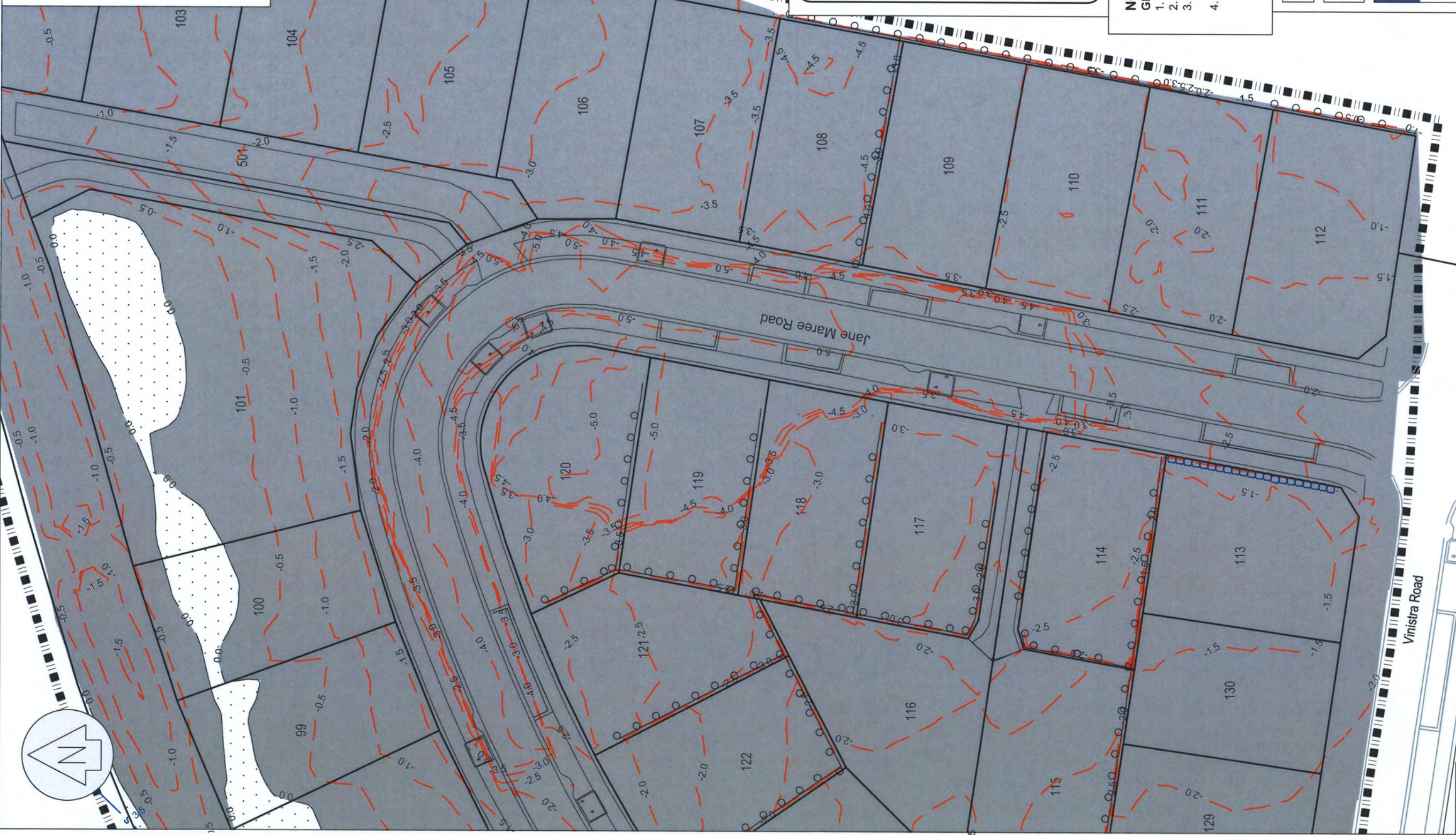
CLIENT Cabra Developments Ltd
Huapai Triangle Sub Precinct A
Stage 1C
Station Road, Huapai

REVISION (DESCRIPTIONS)	NAME	DATE
SURVEYED	BJ	07/2017
DESIGNED	BJ	07/2017
DRAWN	BM	07/2017
CHECKED	TL	07/2017
APPROVED		

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Signed: 11/9/12
 Licensed Cadastral Surveyor

Date: 11/9/12
 Name: Michelle Bain
 Phone: 09 427 0072
 Email: catobolam@catobolam.co.nz



202

Cut to Fill Legend

0	0 Contour
-1.0	Cut Contours & Area
1.0	Fill Contours & Area
1.0	As built Timber Retaining Wall
1.0	Concrete Key Stone Retaining Wall
5.0	Subsoil Drainage & depth (m)

NOTES

GENERAL

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2. Coordinates are in terms of NZTM.
3. All infrastructure is public unless otherwise shown.
4. Cut/fill contours are from the original ground level, to finished ground level as surveyed on 21/07/2017. Excludes undercuts which are shown on plans E608 - E610

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CATO BOLAM CONSULTANTS

SURVEYORS

PLANNERS

ENGINEERS

CATO BOLAM CONSULTANTS LTD
 19 Tamaki Avenue
 P.O. Box 157
 Orewa 0946
 phone 09 427 0072
 fax 09 426 7331
 email catobolam@catobolam.co.nz

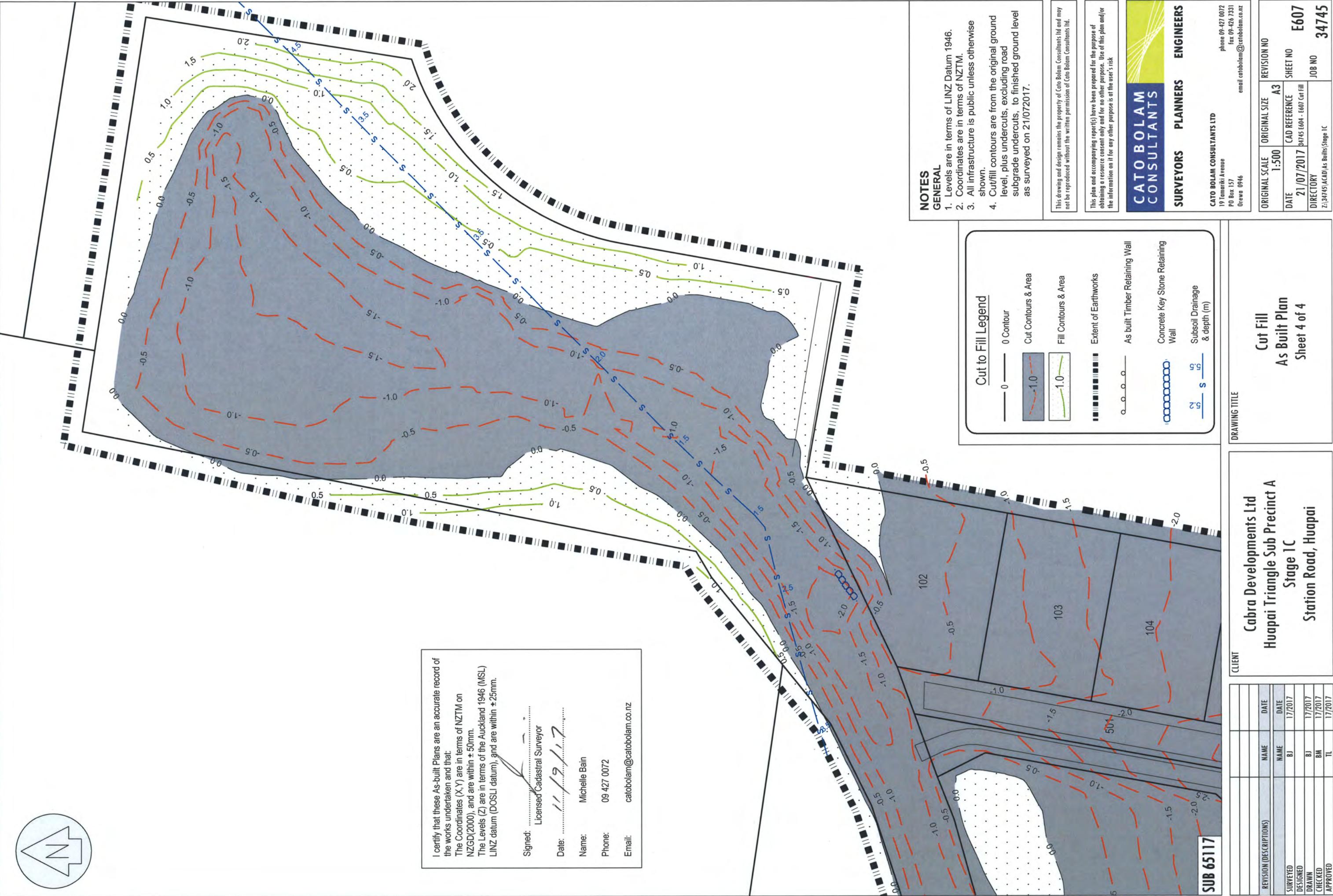
ORIGINAL SCALE 1:500 CAD REFERENCE A3 REVISION NO A3
 DATE 21/07/2017 SHEET NO E606
 DIRECTORY E:\\34745\\ACD\\As Built\\Stage 1C JOB NO 34745

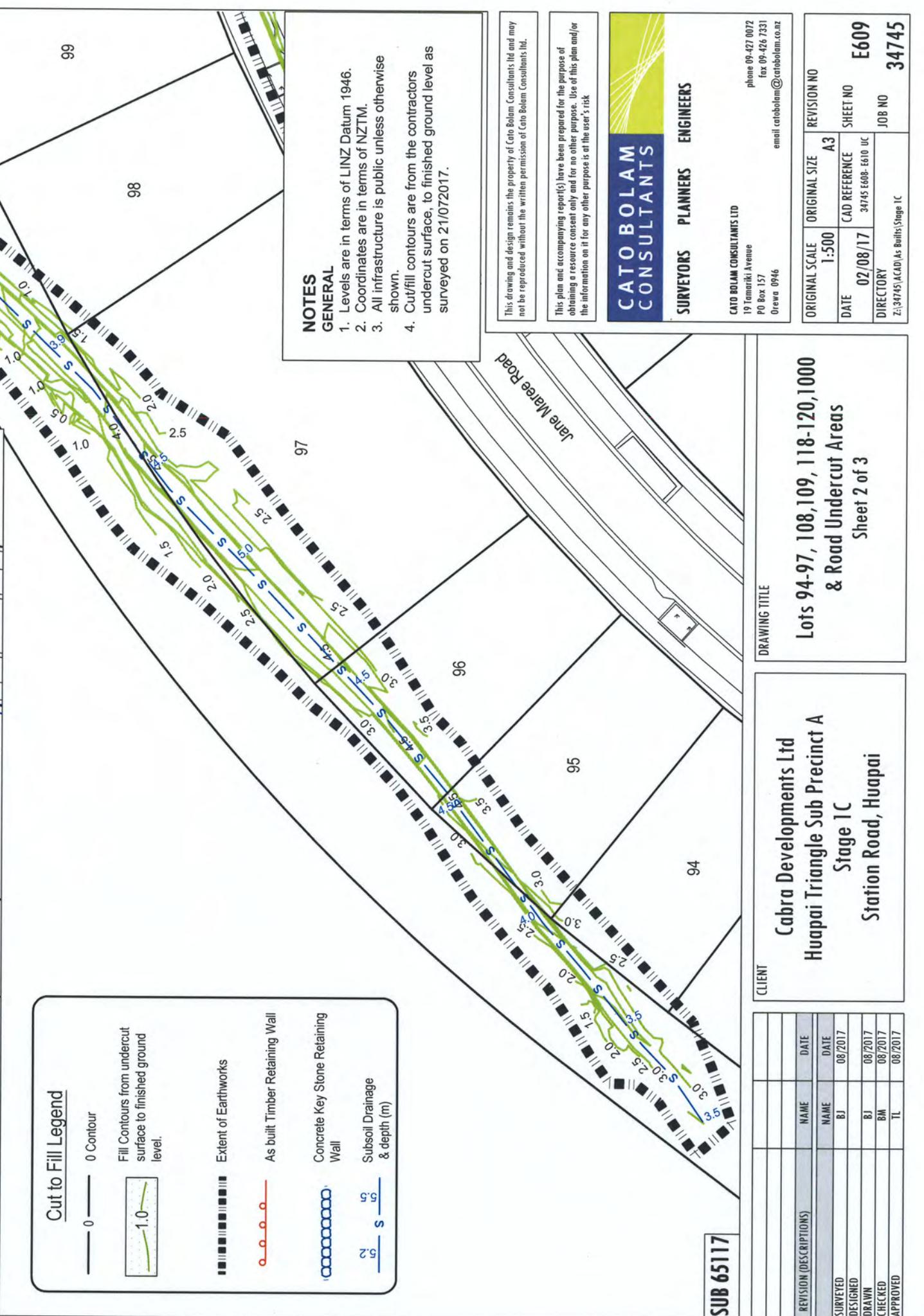
Cut Fill As Built Plan Sheet 3 of 4

DRAWING TITLE

CLIENT
 Cabra Developments Ltd
 Huapai Triangle Sub Precinct A
 Stage 1C
 Station Road, Huapai

REVISION (DESCRIPTIONS)	NAME	DATE
SURVEYED	BJ	07/2017
DESIGNED	BJ	07/2017
DRAWN	BM	07/2017
CHECKED	TL	07/2017
APPROVED		





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 The Levels (Z) are in terms of the Auckland 1946 (MSL) LINZ datum (DOSLI datum), and are within ± 25 mm.

Signed:
 Licensed Cadastral Surveyor

Date: 11/9/17

Name: Michelle Bain

Phone: 09 427 0072

Email: catobolam@catobolam.co.nz



NOTES**GENERAL**

1. Levels are in terms of LINZ Datum 1946.
2. Coordinates are in terms of NZTM.
3. All infrastructure is public unless otherwise shown.

Pavement Formation

Road
 Subgrade - Stabilised with 14kg/m² lime to a depth of 250mm
 Subbase - 250mm GAP65
 Basecourse - 100mm TNZ AP40
 Seal - 30mm DG10 hotmix on grade 4 chisel seal membrane

Kerb & channel

Machine laid low profile mountable kerb

Parking Bays

Subgrade - lime stabilised with 14kg/m² to a depth of 250mm
 175mm 20MPa exposed aggregate concrete with 4kg/m³ black oxide, one layer of 665 mesh on 100mm GAP65 subbase

Footpaths

1.8m - 2.5m wide, 125mm 20MPa concrete on 50mm AP25 bedding

Vehicle Crossings

150mm 20MPa concrete with one layer of 665 mesh on 100mm GAP65 subbase, broom finish.

ROW/ JOAL

150mm 20MPa exposed aggregate concrete with 4kg/m³ black oxide, one layer of 665 mesh on 100mm GAP65 subbase



Refer to sheet E612 for details

200

open channel drain stabilised
& coconut matting

97

96

95

94

93

1000
Stormwater Utility Reserve

98

99

100

101

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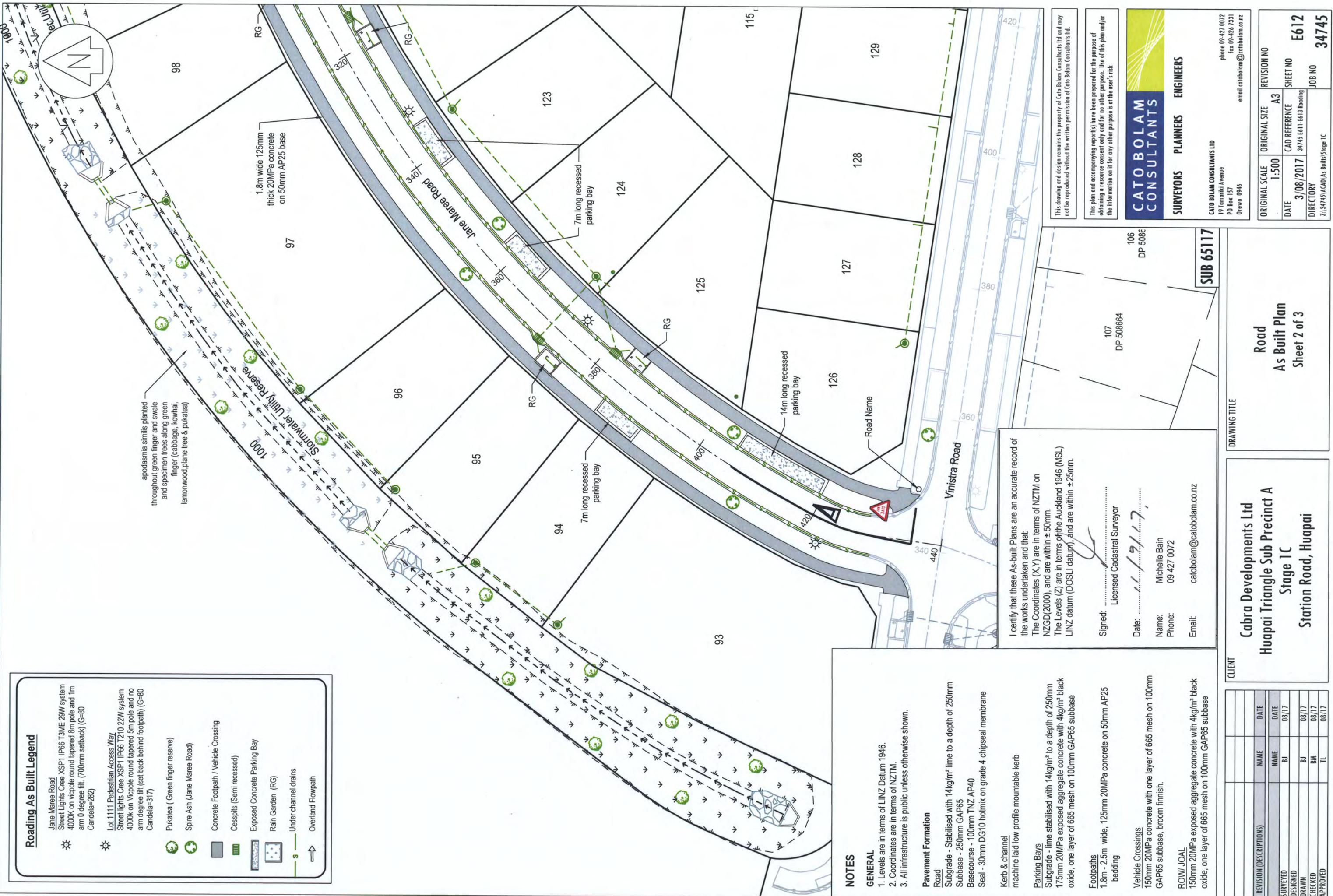
263

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267



NOTES

GENERAL

1. Levels are in terms of LINZ Datum 1946.
2. Coordinates are in terms of NZTM.
3. All infrastructure is public unless otherwise shown.

Pavement Formation

Road
Subgrade - Stabilised with 14kg/m² lime to a depth of 250mm
Subbase - 250mm GAP65
Basecourse - 100mm TNZ AP40
Seal - 30mm DG10 holmix on grade 4 chipseal membrane

Kerb & channel
machine laid low profile mountable kerb

Parking Bays
Subgrade - lime stabilised with 14kg/m² lime to a depth of 250mm
175mm 20MPa exposed aggregate concrete with 4kg/m³ black oxide, one layer of 665 mesh on 100mm GAP65 subbase

Footpaths
1.8m wide, 125mm 20MPa concrete on 50mm AP25 bedding

Vehicle Crossings
150mm 20MPa concrete with one layer of 665 mesh on 100mm GAP65 subbase, broom finish.

ROW/ JOAL
150mm 20MPa exposed aggregate concrete with 4kg/m³ black oxide, one layer of 665 mesh on 100mm GAP65 subbase

97
1.8m wide 125mm thick 20MPa concrete on 50mm AP25 base

97
1.8m wide 125mm thick 20MPa concrete on 50mm AP25 base

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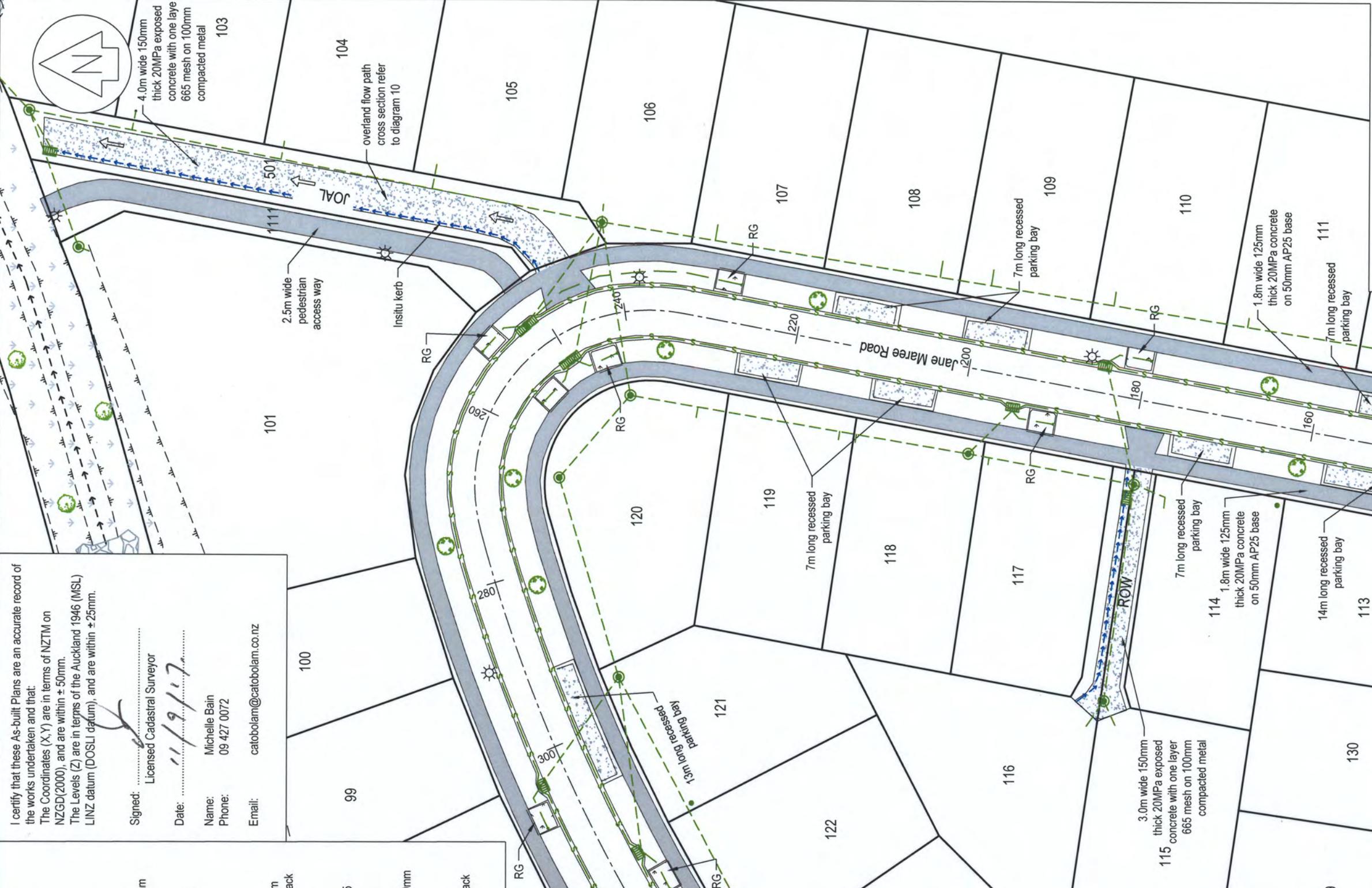
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1.8m wide 125mm thick 20MPa concrete on 50mm AP25 base

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97
1.8m wide 125mm thick 20MPa concrete on 50mm AP25 base

97
1.8m wide 125mm thick 20MPa concrete on 50mm AP25 base



Roading As Built Legend

Jane Maree Road
Street Lights Creek XSP1 IP66 T3ME 29W system
4000K on Vipole round tapered 8m pole and 1m arm 0 Degree tilt. (700mm setback) (G=80
Candela=282)

Lot 1111 Pedestrian Access Way
Street Lights Creek XSP1 IP66 T120 22W system
4000K on Vipole round tapered 5m pole and no arm degree tilt (set back behind footpath) (G=80
Candela=317)

Pukaea (Green finger reserve)

Spiral Ash (Jane Maree Road)

Concrete Footpath / Vehicle Crossing

Cesspits (Semi recessed)

Exposed Concrete Parking Bay

Rain Garden (RG)

Under channel drains

S Overland Flowpath

CLIENT Cabra Developments Ltd

Huapai Triangle Sub Precinct A
Stage 1C
Station Road, Huapai

DRAWING TITLE

Road
As Built Plan
Sheet 3 of 3

SUB 65117

Road Name

460

440

420

400

380

360

340

320

300

280

260

240

220

200

180

160

140

120

100

80

60

40

20

0

100

200

300

400

500

600

700

800

900

1000

1100

1200

1300

1400

1500

1600

1700

1800

1900

2000

2100

2200

2300

2400

2500

2600

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2800

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7300

7400

7500

7600

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8000

8100

8200

8300

8400

8500

8600

8700

8800

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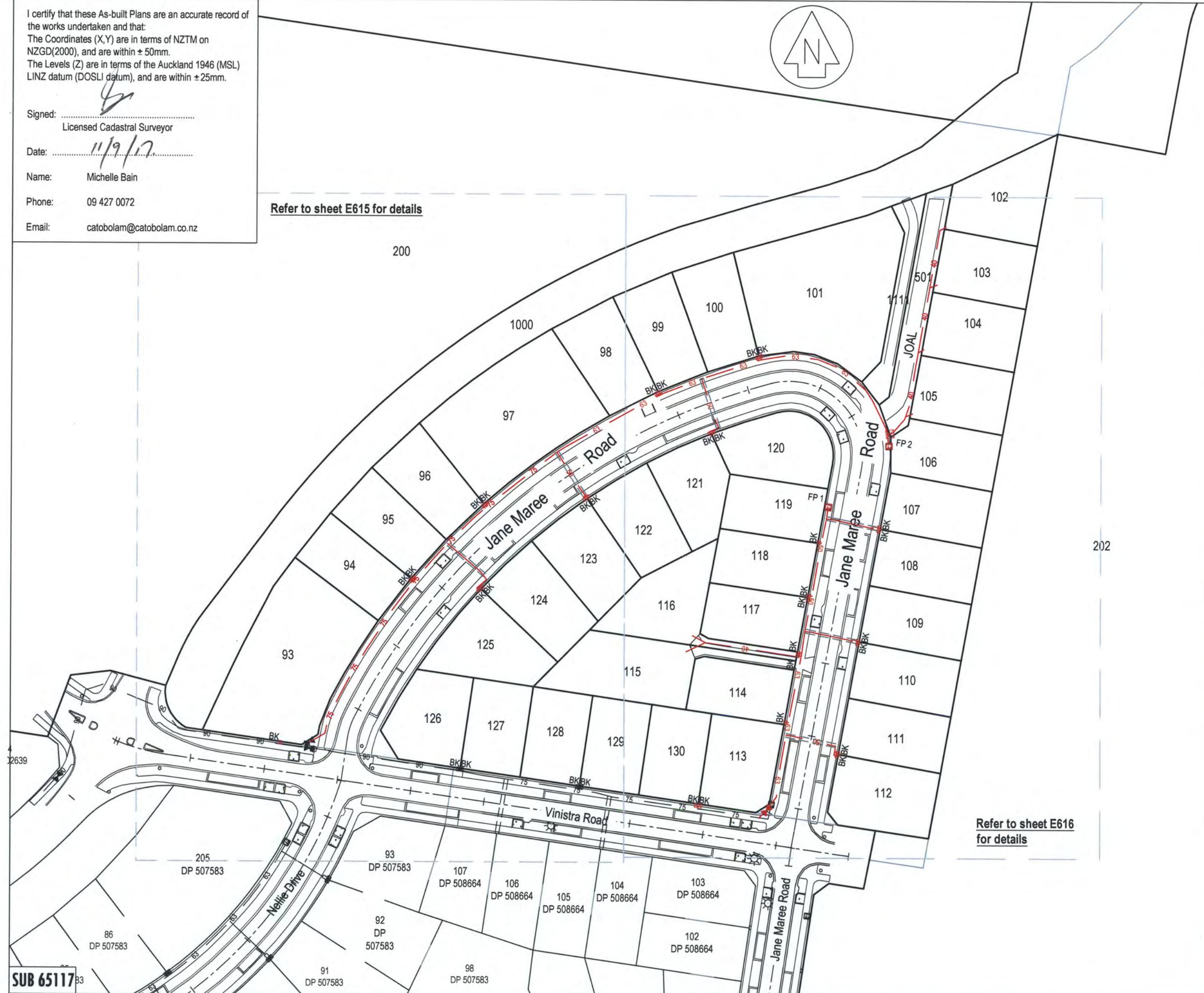
Signed:
Licensed Cadastral Surveyor
Date: 11/9/17
Name: Michelle Bain
Phone: 09 427 0072
Email: catobolam@catobolam.co

Refer to sheet E615 for details



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Asbuilt Legend

Existing	New - Constructed under
	SUB 65117 Stage 1C
75	 PWC Sewer
110	(40,50,63,90,110 outside diameter)
	 50  63  90  110
	Isolation Valve
	Flushing Point
	Boundary Kit
	 FP
	 BK

NOTE

1. Levels are in terms of LINZ Datum.
2. Coordinates are in terms of NZTM.
3. All pipework is PE100 PN16 with SAP7 bedding.
4. Depth of lines are approximately 0.6m below finished ground level and berms. 0.9m under carriageways.

REVISION (DESCRIPTIONS)	NAME	DATE
SURVEYED	BJ	07/2017
DESIGNED		
DRAWN	BJ	08/2017
CHECKED	BM	08/2017
APPROVED	TL	08/2017

**CATO BOLAM
CONSULTANTS**

SURVEYORS PLANNERS ENGINEERS

CATO BOLAM CONSULTANTS LTD
19 Tamariki Avenue
PO Box 157
Orewa 0946

phone 09-427 0072
fax 09-426 7331
am@rotobolam.co.nz

CLIENT Cabra Development Ltd
Huapai Triangle Sub Precinct A
Stage 1C
Station Road, Huapai

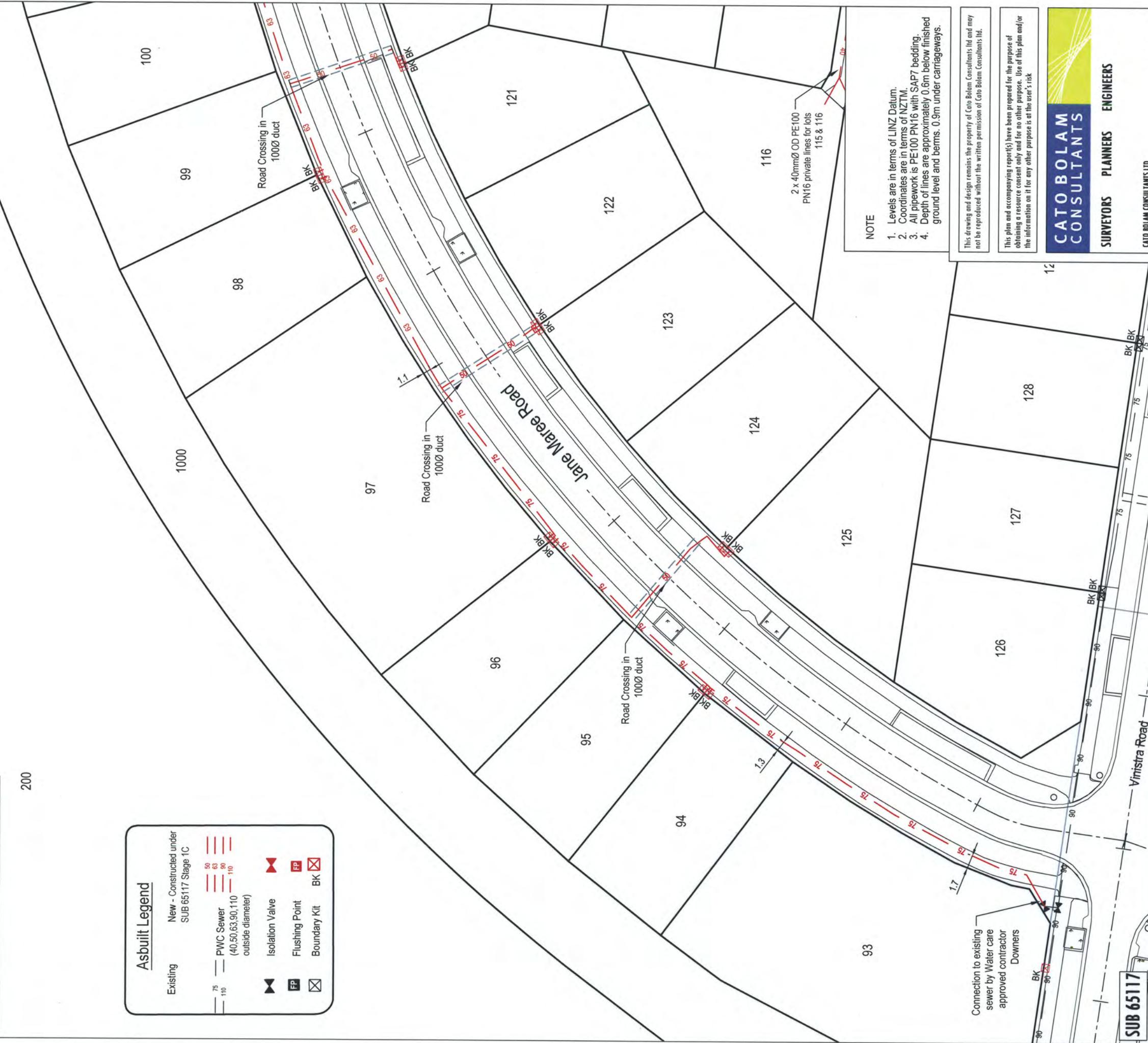
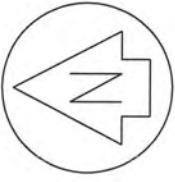
DRAWING TITLE
Wastewater Reticulation
As Built Plan
Sheet 1 of 3

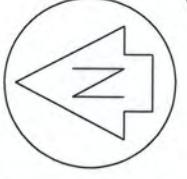
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DATE 04/07/2017	CAD REFERENCE 34745 E614 - E616 LPS	SHEET NO E614
DIRECTORY Z:\34745\ACAD\As Built\Stage 1C		JOB NO 34745

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Signed:
Licensed Cadastral Surveyor
Date: 11/08/17

Name: Michelle Bain
Phone: 09 427 0072
Email: catobolam@catobolam.co.nz





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Signed: Michelle Bain
Date: 11/02/17
Name: Michelle Bain
Phone: 09 427 0072
Email: catobolam@catobolam.co.nz

103

501

1111

104

4 x 40mm \varnothing OD PE100
PN16 private lines for lots
102-105

105

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103

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4 x 40mm \varnothing OD PE100
PN16 private lines for lots
102-105

105

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103

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4 x 40mm \varnothing OD PE100
PN16 private lines for lots
102-105

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4 x 40mm \varnothing OD PE100
PN16 private lines for lots
102-105

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4 x 40mm \varnothing OD PE100
PN16 private lines for lots
102-105

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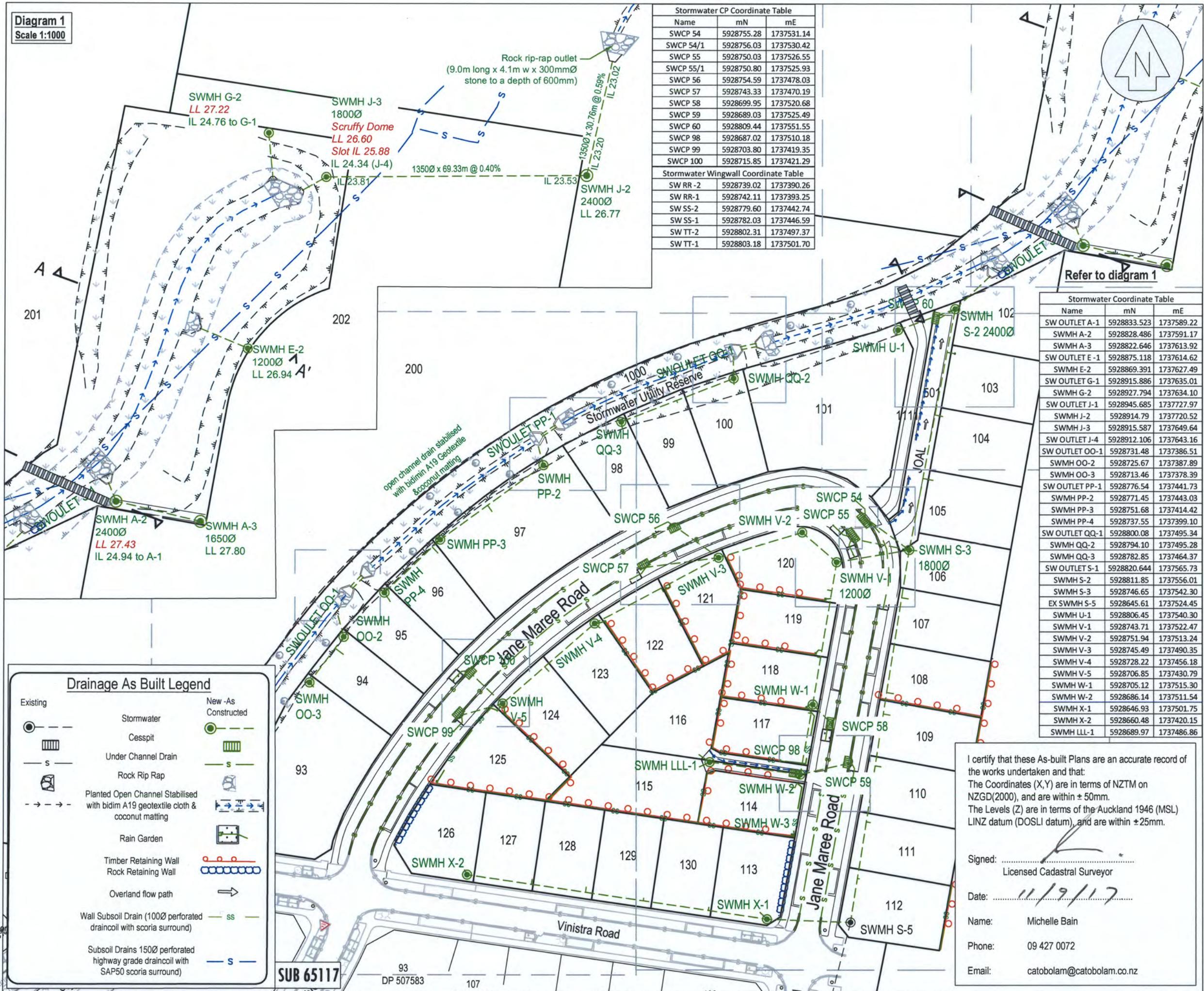
4 x 40mm \varnothing OD PE100
PN16 private lines for lots
102-105

105

0.7

Diagram 1
Scale 1:1000

Scale 1:100



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NOTES GENERAL

1. Levels are in terms of LINZ Datum 1946.
2. Coordinates are in terms of NZTM.
3. All infrastructure is public unless otherwise shown.

STORMWATER

1. All pipes are Class 4 reinforced concrete rubber ring jointed (RCRRJ), unless otherwise shown.
2. All manholes are 1050mmØ concrete flange base and riser, unless otherwise shown.
2. Bedding is H2 type unless otherwise stated.
3. All catchpits are 675mm x 450mm steel grate lids semi recessed unless otherwise shown.
4. All lot connections are 100Ø uPVC SN16 unless otherwise stated.
5. All 100Ø - 150Ø pipes are uPVC SN16.
6. Wall drain cesspits and outlet pipes are private.
7. Information shown in red *italics* are design levels and subject to change

	NAME	DATE
URVEYED	BJ	07/17
ESIGNED	KM	08/16
RAWN	BJ	07/17
HECKED	BM	07/17
PPROVED	TL	07/17

KATO BOLAM CONSULTANTS

SURVEYORS PLANNERS ENGINEERS

ATO BOLAM CONSULTANTS LTD.

9 Tamariki Avenue
0 Box 157
Irewa 0946
phone 09-427 0072
fax 09-426 7331
email cabobolam@cabobolam.co.nz

CLIENT Cabra Development Ltd
Huapai Triangle Sub Precinct A
Stage 1C
Station Road, Huapai

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The Levels (Z) are in terms of the Auckland 1946 (MSL)

Signed: 

Date: 6/26/12

Name: Michelle

Phone: 09 427 0

Email: catchelam@catchelam.co.nz

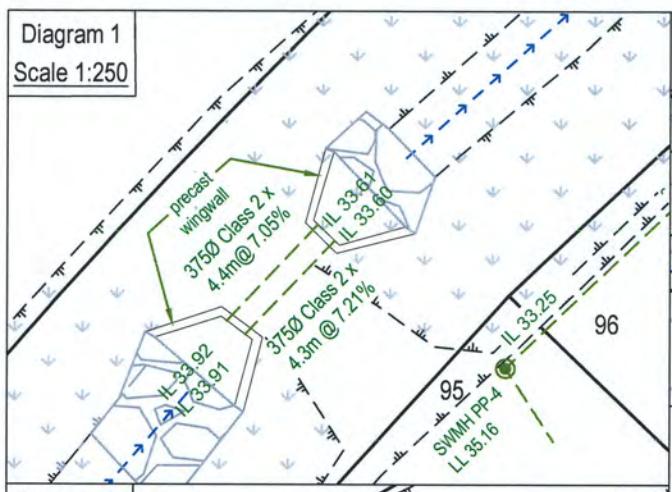
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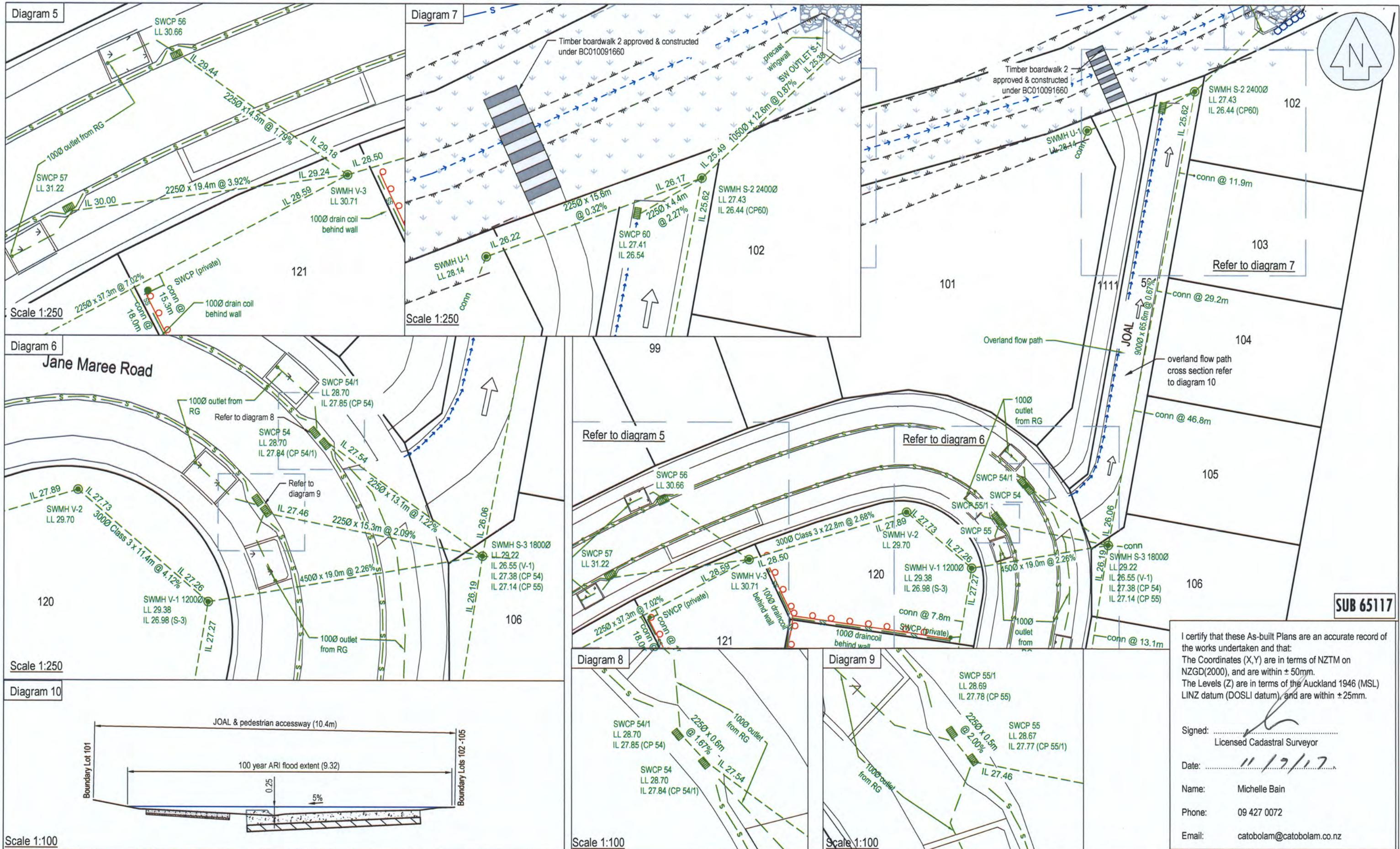
**Stormwater Reticulation
As Built Plan**

RAWING TITLE
**Stormwater Reticulation
As Built Plan
Sheet 1 of 4**

ORIGINAL SCALE 1:1000	ORIGINAL SIZE A3	REVISION NO
DATE 04/07/17	CAD REFERENCE 34745 E617-E621 SW RETIC	SHEET NO E617
DIRECTORY Z:\34745\ACAD\As-Builts\Stage 1C		JOB NO 34745

Diagram 1
Scale 1:250







Refer to E617 for Legend

the works undertaken and that:
The Coordinates (X, Y) are in terms of NZTM on NZGD(2000), and are within $\pm 50\text{mm}$.
The Levels (Z) are in terms of the Auckland 1946 (MSL) LINZ datum (DOSLU datum), and are within $\pm 25\text{mm}$.

Signed: Licensed Cadastral Surveyor
Date: 11/17/17

Phone: 09 427 0072
Email: catobolam@catobolam.co.nz

CATO BOLAM
CONSULTANTS

111

PO Box 157
Orewa 0946
original ccw
original csw
original cmo
email cateboleham@entoblolam.co.nz
fax 09-426 7331

DATE	04/07/17	CAD REFERENCE	SHEET NO	E620
DIRECTORY	Z:\34145\ACAD\As Built\Stage 1C			
JOB NO				34745
REV				

Stormwater Reticulation
As Built Plan
Sheet 4 of 4

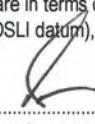
111

phone 09-427 0072
fax 09-426 7331
@catobolam.co.nz

Cabra Development Ltd
Huapai Triangle Sub Precinct A
Stage 1C
Station Road, Huapai

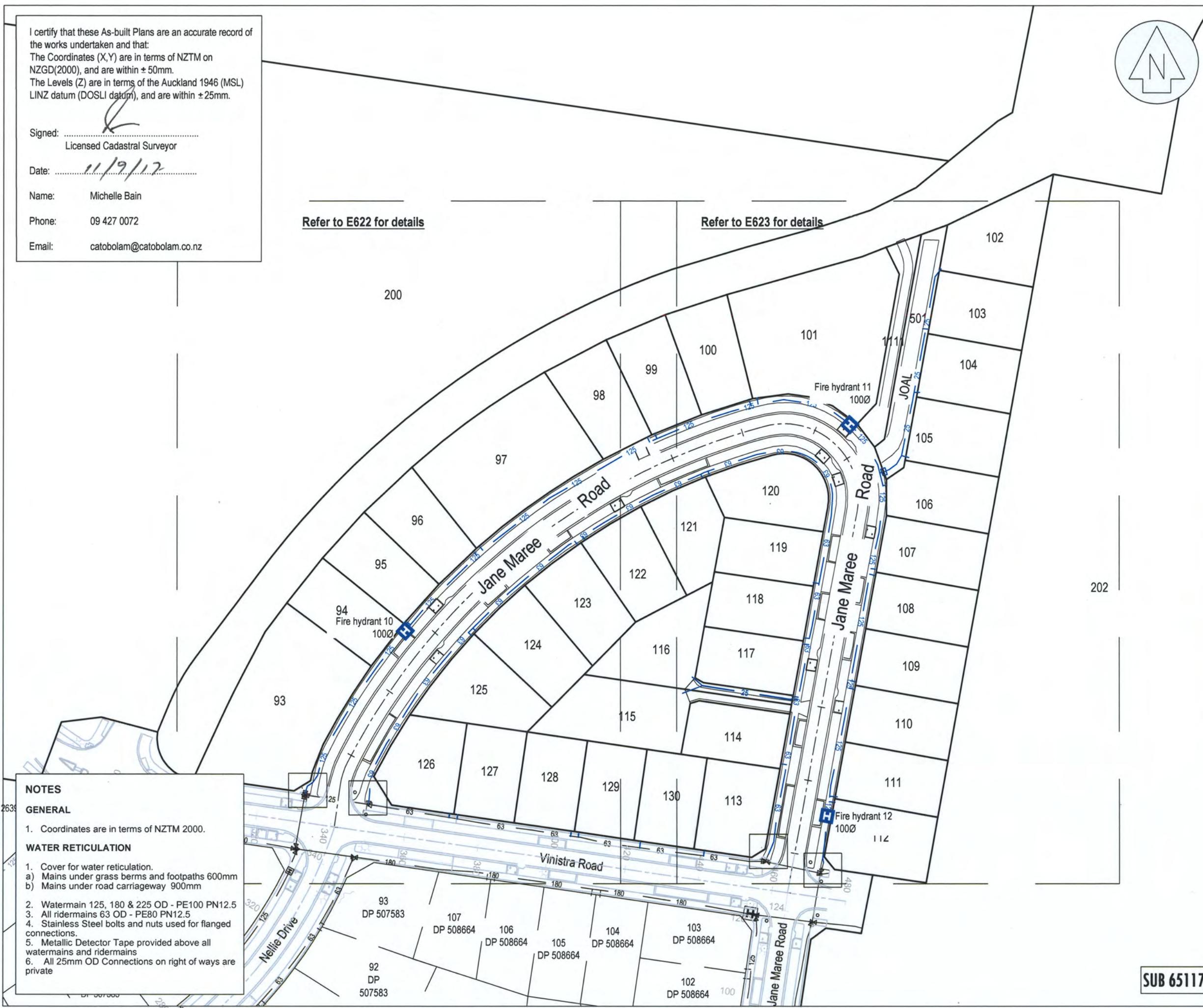
REVISION (DESCRIPTION)	NAME	DATE
SURVEYED	BJ	07/17
DESIGNED	KM	08/16
DRAWN	BJ	07/17
CHECKED	BM	07/17
APPROVED	TL	07/17

I certify that these As-built Plans are an accurate record of the works undertaken and that:
 The Coordinates (X,Y) are in terms of NZTM on NZGD(2000), and are within ±50mm.
 The Levels (Z) are in terms of the Auckland 1946 (MSL) LINZ datum (DOSLI datum), and are within ±25mm.

Signed: 
 Licensed Cadastral Surveyor
 Date: 11/9/17
 Name: Michelle Bain
 Phone: 09 427 0072
 Email: catobolam@catobolam.co.nz

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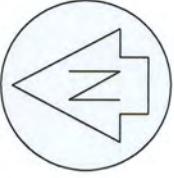
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I certify that these As-built Plans are an accurate record of the works undertaken and that:
 The Coordinates (X, Y) are in terms of NZTM on NZGD2000, and are within ± 50 mm.
 The Levels (Z) are in terms of the Auckland 1946 (MSL) LINZ datum (DOSL) datum, and are within ± 25 mm.

Signed:
 Licensed Cadastral Surveyor
 Date: 11/12/17
 Name: Michelle Bain
 Phone: 09 427 0072
 Email: catobolam@catobolam.co.nz

200



100
 Lot connection lots 100 &

99

Lot connection lots 98 &

98

Lot connections lots 98 & 99

97

Lot connections lots 122 & 123

96

Lot connections lots 124 & 125

95

Lot connections lots 96 & 97

94

Lot connections lots 94 & 95

93

Lot connections lots 116 & 115

116

Lot connection lot 116

115

Lot connection lot 115

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Lot connection lot 121

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Lot connection lot 122

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Lot connection lot 123

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Lot connection lot 195

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Lot connection lot 196

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Lot connection lot 197

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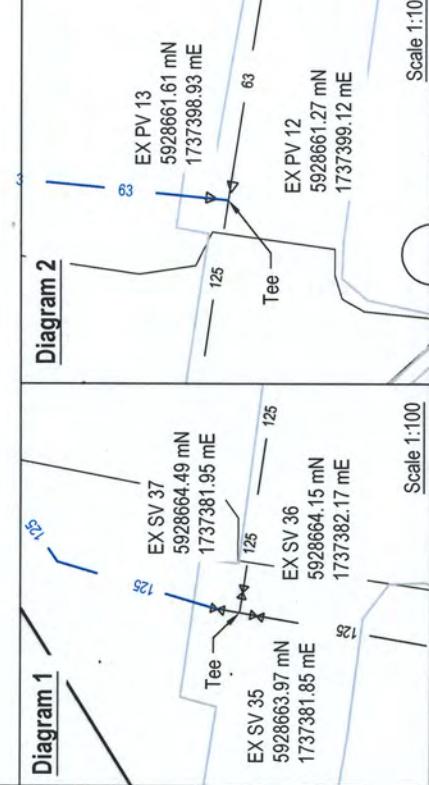
Lot connection lot 198

199

Lot connection lot 199

200

Lot connection lot 200



SUB 65117

Water Reticulation Legend

Existing

New - constructed under SUB 65117

Watermain (OD Ø)

Sluice Valve

Peet Valve

Tapping bands for lot connections

Fire Hydrant

H

Existing

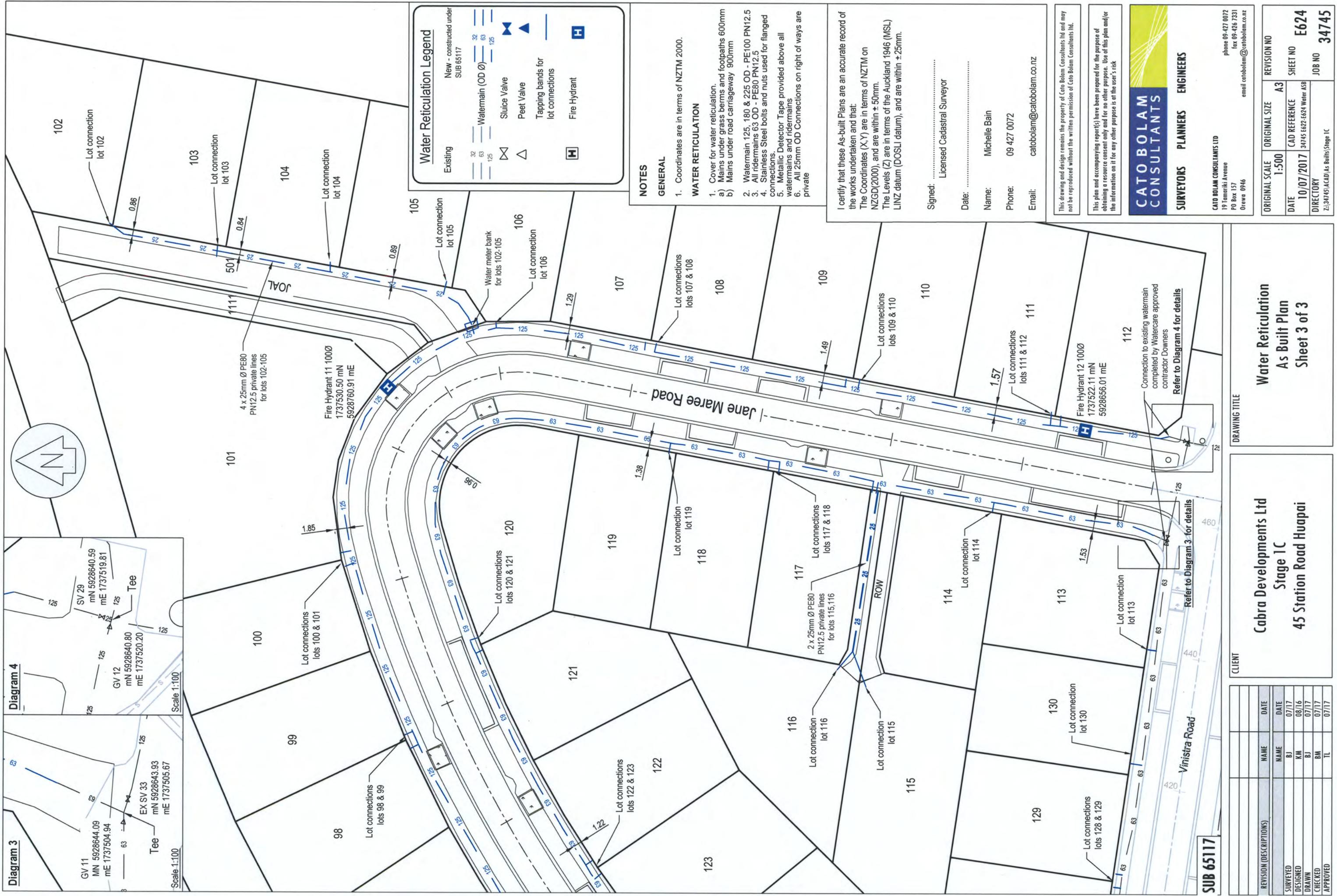
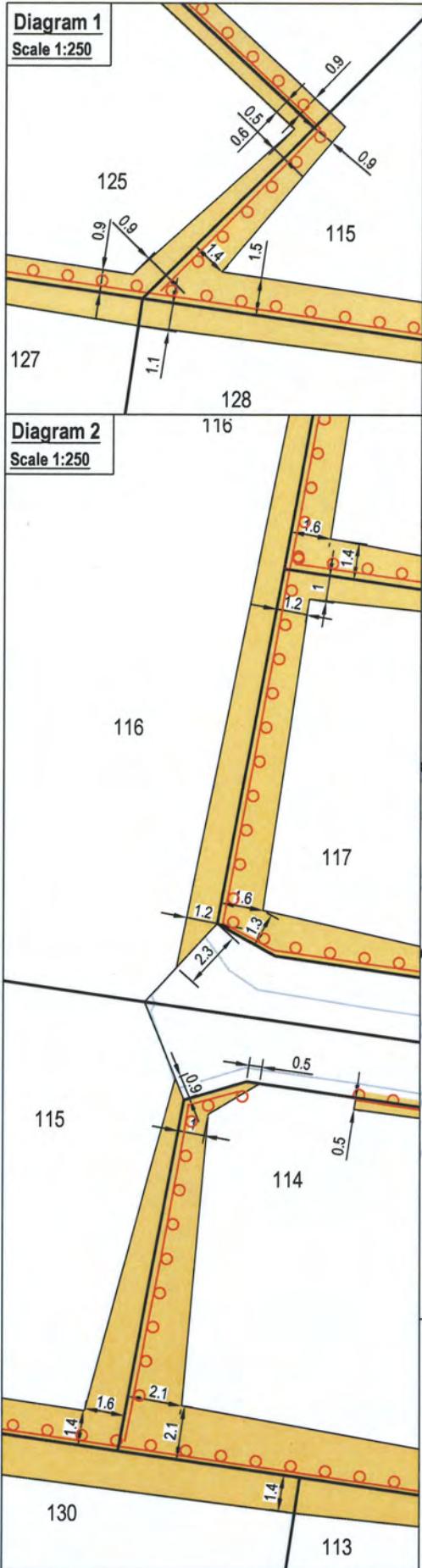
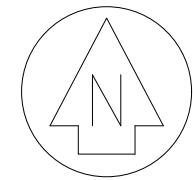


Diagram 1
Scale 1:250





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As Built Earthworks Legend

- Pipeline 45° Zone of Influence Area from invert
- Catchpits
- Stormwater
- Offset dimension from boundary as shown on plan



Lot 101 area around manhole added	BJ	09/17
REVISION (DESCRIPTIONS)	NAME	DATE
SURVEYED	NAME	DATE
DESIGNED	BJ	07/17
DRAWN	KM	09/14
CHECKED	BJ	08/17
APPROVED	BM	08/17
	TL	08/17

CATO BOLAM CONSULTANTS 
SURVEYORS PLANNERS ENGINEERS
CATO BOLAM CONSULTANTS LTD
19 Tamaki Avenue
PO Box 157
Orewa 0946
phone 09-427 0072
fax 09-426 7331
email catabolam@catabolam.co.nz

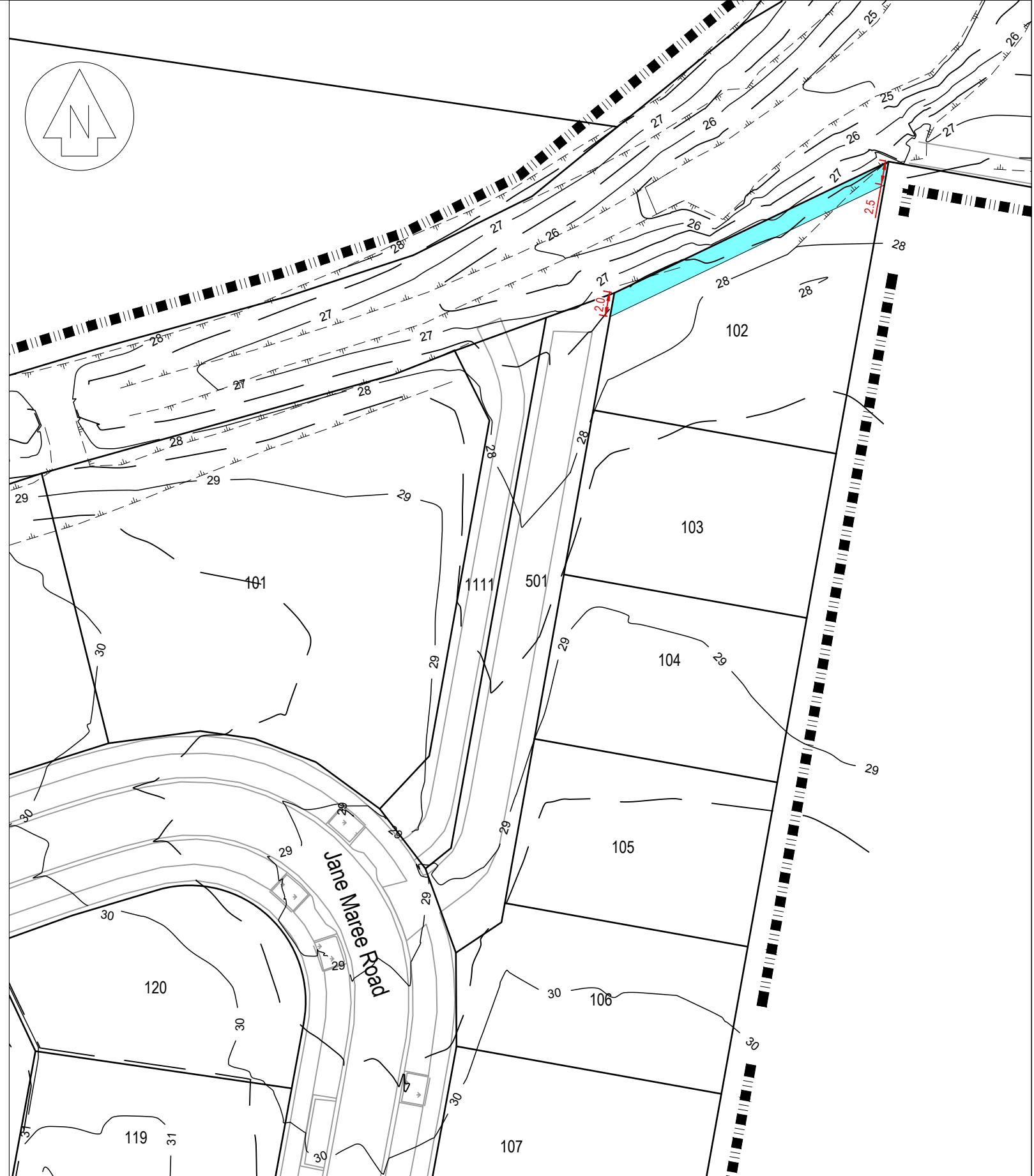
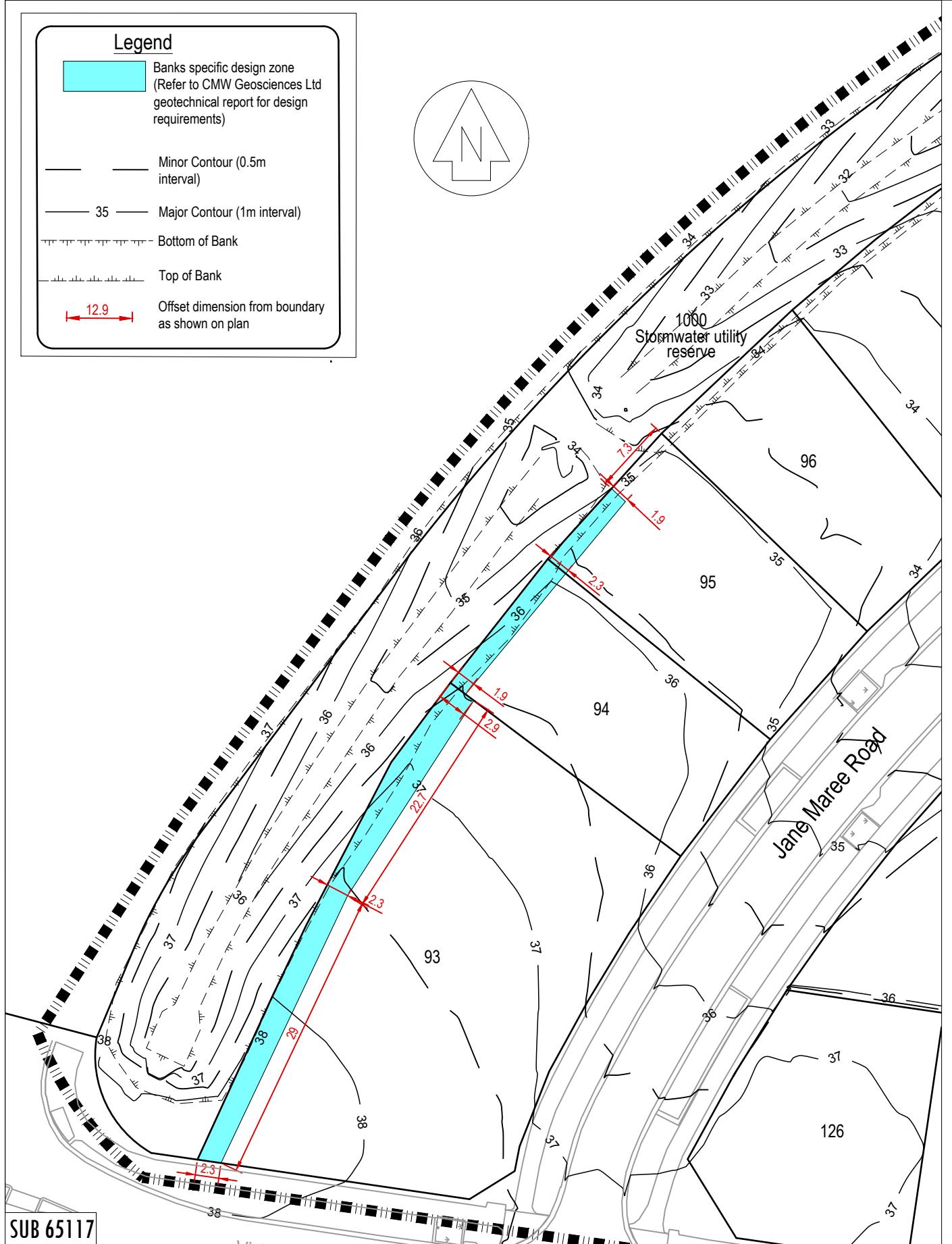
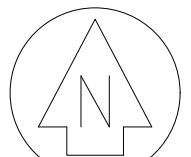
CLIENT Cabra Development Ltd
Huapai Triangle Sub Precinct A
Stage 1C
Station Road, Huapai

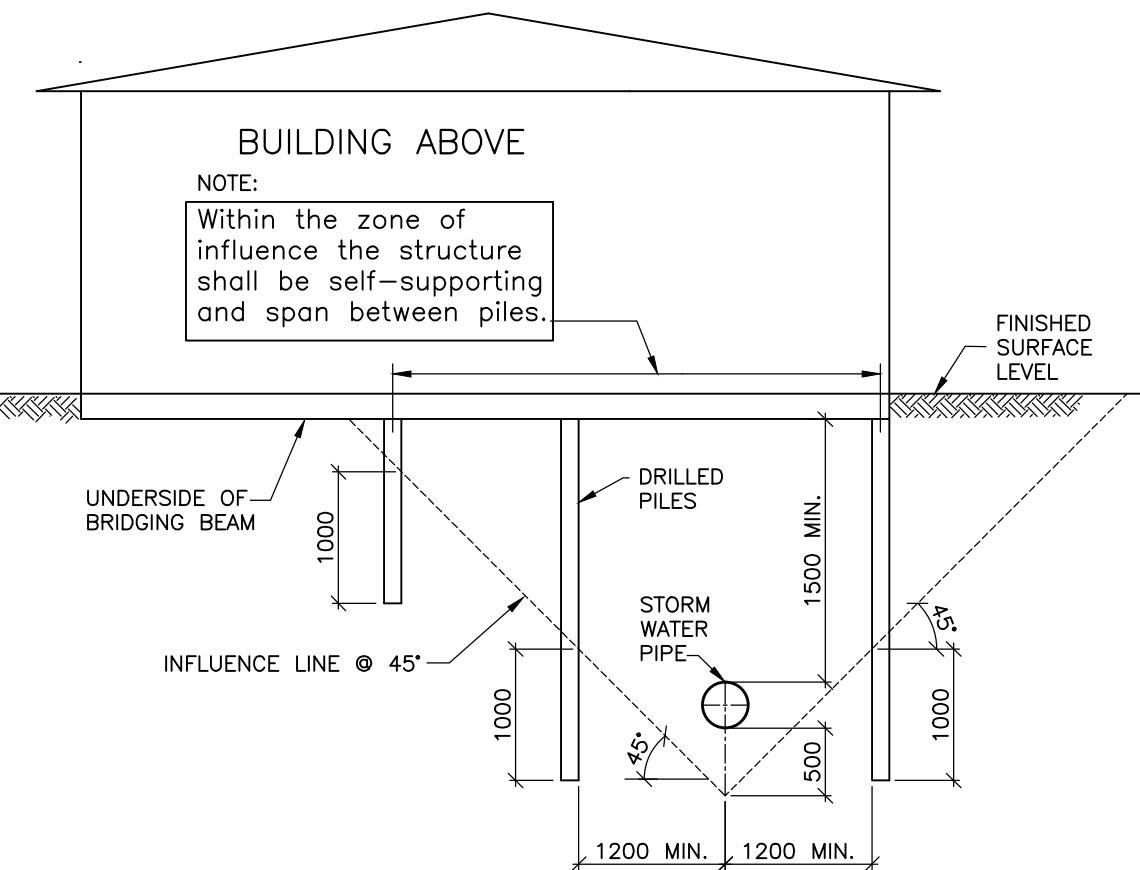
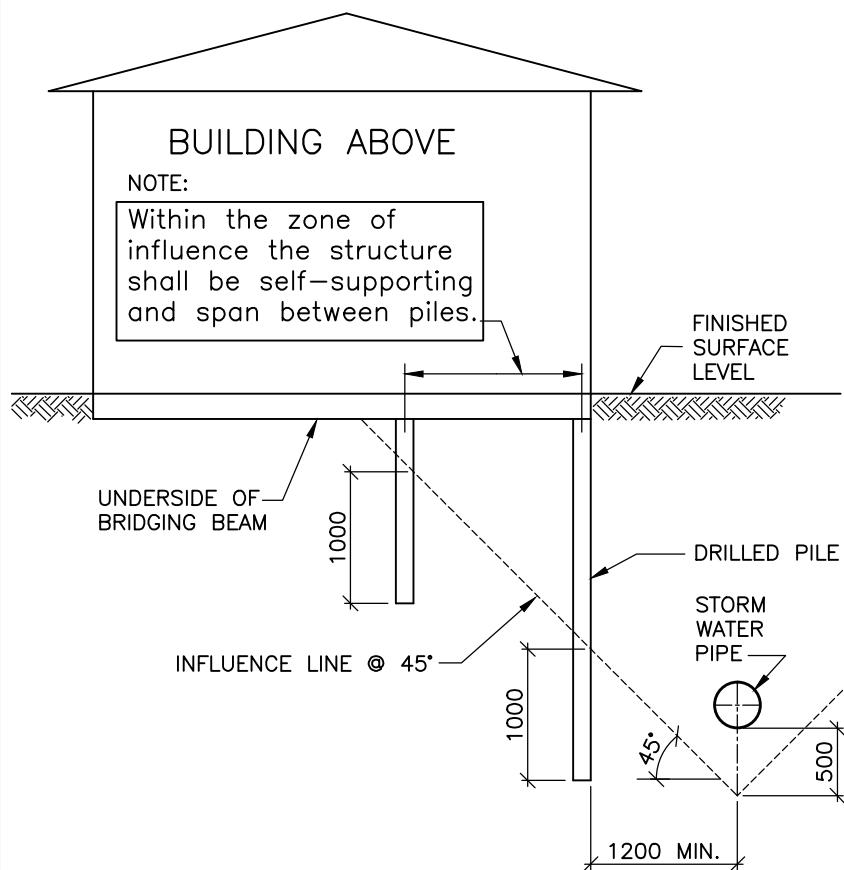
DRAWING TITLE
Drainage Zone of Influence Plan

ORIGINAL SCALE	ORIGINAL SIZE	REVISION NO
1:1000	A3	R1
DATE	CAD REFERENCE	SHEET NO
04/08/17	34745 E626 SW ZONE	E626
DIRECTORY	JOB NO	
Z:\34745\Acad\As built	34745	

Legend

	Banks specific design zone (Refer to CMW Geosciences Ltd geotechnical report for design requirements)
	Minor Contour (0.5m interval)
	Major Contour (1m interval)
	Bottom of Bank
	Top of Bank
	Offset dimension from boundary as shown on plan





NOTES

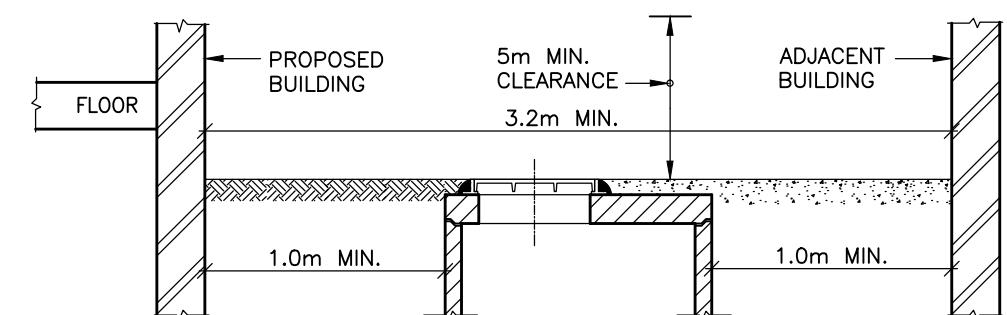
1. The information on this page is intended to show examples of typical scenarios and should be used for general guidance purposes only. Significant variations on a site by site basis are to be expected and it is in no way implied that meeting any of the above will guarantee approval.
2. Requirements for foundation design etc apply to both sides of pipe.
3. No driven piles are permitted within 10m of brick Stormwater Structures, or within 5m of all other Stormwater Structures.
4. Specific approval is required from Auckland Council for driven piles in partially drilled holes, within the 5m/10m zone.
5. Piles that may be required to resist horizontal forces will require specific design.
6. Pile/Footing location point must be below 45° "Zone of Influence".
7. All Manholes shall have 24 hours unobstructed access.
8. Manholes in basements, or where sufficient clearance is unavailable, are not permitted.
9. All pipe buildovers will require approval by Auckland Council.
10. Refer to section 4.3.21 for pipe buildover requirements.

"BUILD CLOSE" NOTES:

1. Specific approval is required from Auckland Council if building adjacent to pipes, larger than 375mm internal diameter, or greater than 3.0m of depth.
2. Building to be outside all overland flow paths and floodplains.
3. Pile constructed to a depth of 1.0m below influence line.
4. Outside zone of influence, normal foundation requirements apply.

"BUILD OVER" NOTES:

1. Applies to stormwater pipes 375mm nominal diameter or less.
2. Bridging over pipes larger than 375mm nominal diameter is NOT allowed under any circumstances.
3. Pile constructed to a depth of 1.0m below influence line.
4. Outside zone of influence, normal foundation requirements apply.
5. Bridging is NOT allowed over pipes where clear vertical separation distance from top of pipe to underside of bridging beam is less than 1.5m



MANHOLE CONSTRUCTION CLEARANCE

Appendix C

Laboratory Test Data

**CONSTANT HEAD PERMEABILITY
TEST REPORT**



Project : **57 Nobilo Road**
 Location : **57 Nobilo Road**
 Client : **CMW Geosciences(NZ) Ltd**
 Contractor : **Not Stated**
 Sampled by : **Not Stated**
 Date sampled : **07/12/16**
 Sampling Method: ***Pushtube**
 Sample Description: **Fill**
 Sample Condition: **As Received**
 Sample Ref / Depth: **Sample 1 / Not Stated**

Project No :	1-LA051.16
Lab No :	AL854/1
Client Project No :	AKL2016_0331
Purchase Order No:	AKL830

SOIL PROPERTIES

Sample Reference	Sample 1
Depth(m)	Not Stated
Specimen length (mm)	120.1
Specimen diameter (mm)	60.3
Specimen mass (g)	635.0
Pre test water content (%)	30.4
Wet density (t/m ³)	1.852
Dry density (t/m ³)	1.420
Post test water content (%)	31.8

PERMEABILITY TEST RESULT

See Notes below	Head (kPa)	Permeability (m/s)
	20	3.65×10^{-10}
	40	2.80×10^{-10}
	60	2.34×10^{-10}

Test Method	Notes
Permeability Test Based on : JBS 1377 1990 Part 6 Water Content : NZS 4402 : 1986 Test 2.1	-Sample was tested using the triaxial test apparatus to enable back pressure saturation of the test specimens.

Date tested : **08-17/12/16** *Sampling is not covered by IANZ Accreditation. Results apply only to sample tested.
This report may only be reproduced in full

Date reported : **22/12/16**

IANZ Approved Signatory :

Thirushen Pillay
Designation : **Senior Civil Engineering Technician**
Date : **22/12/2016**



All tests reported herein have been performed in accordance with the laboratory's scope of accreditation

**CONSTANT HEAD PERMEABILITY
TEST REPORT**



Project : **57 Nobile Road**
 Location : **57 Nobile Road**
 Client : **CMW Geosciences(NZ) Ltd**
 Contractor : **Not Stated**
 Sampled by : **Not Stated**
 Date sampled : **07/12/16**
 Sampling Method: ***Pushtube**
 Sample Description: **Fill**
 Sample Condition: **As Received**
 Sample Ref / Depth: **Sample 2 / Not Stated**

Project No :	1-LA051.16
Lab No :	AL854/2
Client Project No :	AKL2016_0331
Purchase Order No:	AKL830

SOIL PROPERTIES	
Sample Reference	Sample 2
Depth(m)	Not Stated
Specimen length (mm)	119.9
Specimen diameter (mm)	60.5
Specimen mass (g)	633.8
Pre test water content (%)	30.9
Wet density (t/m ³)	1.839
Dry density (t/m ³)	1.405
Post test water content (%)	31.0

PERMEABILITY TEST RESULT			
<i>See Notes below</i>	Head (kPa)	Permeability (m/s)	
	20	4.14 x 10⁻¹⁰	
	40	5.67 x 10⁻¹⁰	
	60	1.17 x 10⁻¹⁰	

Test Method	Notes
Permeability Test Based on : JBS 1377 1990 Part 6 Water Content : NZS 4402 : 1986 Test 2.1	-Sample was tested using the triaxial test apparatus to enable back pressure saturation of the test specimens.

Date tested : **08-17/12/16** *Sampling is not covered by IANZ Accreditation. Results apply only to sample tested.
This report may only be reproduced in full

Date reported : **22/12/16**

IANZ Approved Signatory :

Designation : **Thirushen Pillay**
Date : **22/12/2016**



All tests reported herein have been performed in accordance with the laboratory's scope of accreditation

Report No: 17 0145 00
Page: 1 of 1**DETERMINATION OF THE LIQUID LIMIT & LINEAR SHRINKAGE
TEST METHOD NZS 4402 : 1986 TEST 2.2 & 2.6**

Job: **45 Station Road**
Date of order: 31.05.17
Sample method: -
Sample By: Client Sample Origin: -
 Sample Description: -
 Date: 30.5.17

Test Details :

Test performed on : Whole Sample
History : Natural

Sample No.	Location	Depth (m)	Liquid Limit	Linear Shrinkage	Natural Water Content (%)
342F	Lot 103	-	102	21	54.5
343F	Lot 124	-	96	23	41.6
344F	Lot 120	-	100	25	34.5

Comments :

Tested By: EC Date : 9.6.17
Calculated By : EC Date : 12.6.17
Checked By : EC Date : 13.6.17

Appendix D

Field Test Data

LF11 Rev 4 Soil Field Density NDM Direct Transmission with VSS Report

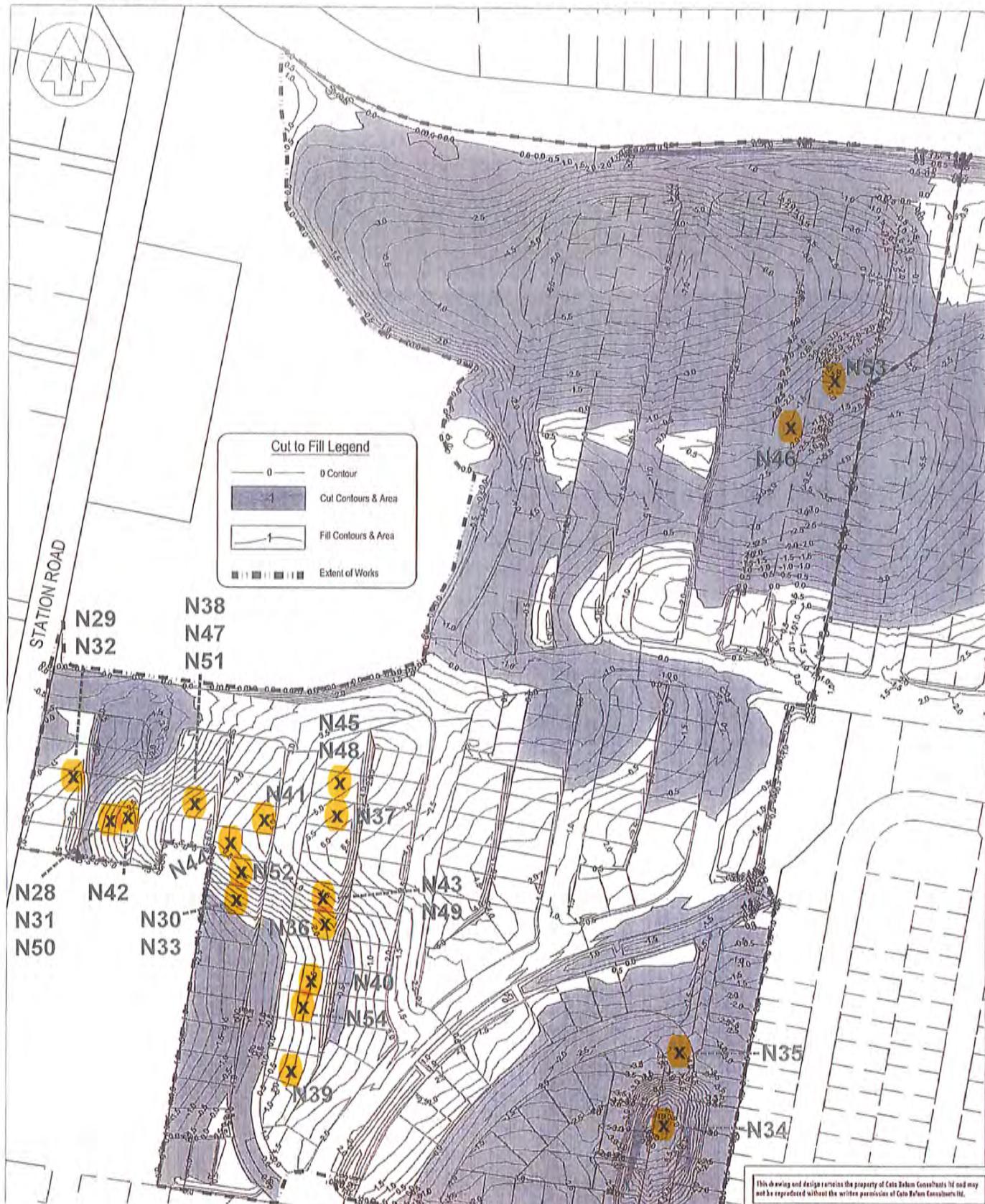
Auckland Laboratory
CMW Geosciences (NZ) Limited
Building C, 9 Piermark Drive, Rosedale, NZ 0632
PO Box 300206, Albany, Auckland, NZ 0752
Phone: +64 (09) 4144 632

Project: 45 Station Road, Huapai Project No: AKL2016_0634 Location: Huapai Report No: AKL2016_0634LAD Rev.0 Report Date: 15/02/2017 Client: Cabra Developments Limited Client Address: Client Reference:												Test Methods:		Notes:									
												NZS 4402.2.1:1986		Solid Density: Assumed		Testing Locations Selected By: CMW Field Staff							
IANZ ACCREDITED LABORATORY																Tests indicated as not accredited are outside the scope of the laboratory's accreditation		Measurements marked * are not accredited and are outside the scope of the laboratories accreditation					
Date Sampled	Sample No.	Test Location	Soil Description	In-situ Vane Shear Strengths				Field and Laboratory Testing Data								Comments							
				Test 1 (kPa)	Test 2 (kPa)	Test 3 (kPa)	Test 4 (kPa)	Ave.	Gauge Wet Density (t/m³)	Gauge Dry Density (t/m³)	Gauge Water Content (%)	Gauge Air Voids (%)	Gauge Probe Depth	Oven Water Content (%)	Solid Density (t/m³) *	Oven Dry Density (t/m³)	Calculated Air Voids (%) *						
12/01/2017	N28	Lot 9	CLAY	97	124	138	135	124	1.7523	1.2313	42.3	2.18	300	35.5	2.7	1.30	5.2	Failed					
	N29	Lot 3	CLAY	167	189	>189	>189	>184	1.7401	1.2240	42.2	2.94	300	39.0	2.7	1.26	4.8						
	N30	Lot 20	CLAY	138	132	167	140	144										Failed					
13/01/2017	N31	Lot 9	CLAY	189	189	>189	>189	>189	1.7241	1.2262	40.6	4.68	300	41.6	2.7	1.22	4.3	Re-test of N28					
	N32	Lot 3	CLAY	>189	>189	189	189	>189	1.7342	1.2154	42.7	2.99	300	50.0	2.7	1.16	-0.6						
	N33	Lot 20	CLAY	178	170	189	183	180	1.7505	1.2899	35.7	6.07	300	36.1	2.7	1.28	5.9	Re-test of N30					
16/01/2017	N34	Refer to site plan	CLAY	189	189	>189	>189	>189	1.8756	1.4034	33.6	0.70	300	32.9	2.7	1.42	1.3						
	N35	Refer to site plan	CLAY	178	175	189	189	183	1.7740	1.3549	30.9	7.82	300	32.3	2.7	1.34	7.0						
	N36	Lot 39	CLAY	UTP	UTP	UTP	UTP	UTP	1.7281	1.2764	35.4	7.46	300	30.7	2.7	1.32	10.0						
	N37	Lot 43	CLAY	UTP	UTP	UTP	UTP	UTP	1.7351	1.2784	35.7	6.88	300	37.1	2.7	1.26	6.2						
	N38	Lot 12	CLAY	UTP	UTP	UTP	UTP	UTP	1.8507	1.3499	37.1	-0.19	300	43.6	2.7	1.28	-3.9						
18/01/2017	N39	Lot 37	CLAY	UTP	UTP	UTP	UTP	UTP	1.8490	1.4097	31.2	3.76	300	31.1	2.7	1.42	3.9						
	N40	Lot 34	CLAY	183	189	189	>189	>188	1.7472	1.2828	36.2	5.95	300	34.9	2.7	1.30	6.9						
	N41	Lot 17	CLAY	127	140	119	121	127	1.6636	1.1636	44.0	5.64	300	35.0	2.7	1.24	11.0	Failed					
	N42	Lot 9	CLAY	>189	>189	>189	>189	>189	1.7471	1.2521	39.5	4.01	300	46.0	2.7	1.20	0.6						
25/01/2017	N43	Lot 40	CLAY	>189	>189	UTP	UTP	>189	1.7865	1.3862	28.9	8.54	300	26.7	2.7	1.40	10.0						
	N44	Lot 18	CLAY	186	189	>189	>189	>188	1.8318	1.3358	37.1	0.81	300	38.8	2.7	1.32	-0.1	Re-test of N41					
	N45	Lot 44	CLAY	173	178	189	>189	>182	1.7411	1.2510	39.2	4.55	300	37.3	2.7	1.26	5.8						
27/01/2017	N46	Refer to site plan	CLAY	178	146	151	165	160	1.7280	1.2275	40.8	4.38	200	45.5	2.7	1.18	2.0						
31/01/2017	N47	Lot 12	CLAY	UTP	UTP	UTP	UTP	UTP	1.7523	1.2719	37.8	4.74	300	39.3	2.7	1.26	3.9						
	N48	Lot 44	CLAY	UTP	UTP	UTP	UTP	UTP	1.7986	1.4142	27.2	9.10	300	34.6	2.7	1.34	4.3						
	N49	Lot 40	CLAY	UTP	UTP	UTP	UTP	UTP	1.7970	1.3901	29.3	7.74	300	35.3	2.7	1.32	3.9						
7/02/2017	N50	Lot 9	CLAY	UTP	UTP	UTP	UTP	UTP	1.7613	1.2824	37.3	4.50	300	44.4	2.7	1.22	0.7						
	N51	Lot 12	CLAY	UTP	UTP	UTP	UTP	UTP	1.7743	1.2653	40.2	2.12	300	52.4	2.7	1.16	-4.1						
	N52	Lot 19	CLAY	UTP	UTP	UTP	UTP	UTP	1.7893	1.3263	34.9	4.48	300	33.2	2.7	1.34	5.6						
	N53	Refer to site plan	CLAY	UTP	UTP	UTP	UTP	UTP	1.7614	1.2984	35.7	5.51	300	34.4	2.7	1.32	6.4						
13/02/2017	N54	Lot 36	CLAY	189	189	181	>189	>187	1.7475	1.2313	41.9	2.66	300	46.5	2.7	1.20	0.4						

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Created By: TG Date: 13/01/2017
Checked By: TG Date: 15/02/2017
Authorised Signatory:  Date: 15/2/2017

Page: 1 of 2



CMW Geosciences

B1 REVISED FOR AMENDED DESIGN		KM	05/2/2016
REVISION (DESCRIPTIONS)		NAME	DATE
SURVEYED			
DESIGNED	KM	15/11/2016	
DRAWN	KM	15/11/2016	
CHECKED			
APPROVED			

CABRA DEVELOPMENTS LTD
45 STATION ROAD,
HUAPAI



DRAWING TITLE

CUT AND FILL DEPTH
CONTOURS PLAN

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**CATO BOLAM
CONSULTANTS**

SURVEYORS PLANNERS ENGINEERS

CATO BOLAM CONSULTANTS LTD
79 Temaki Avenue
PO Box 157
Glen Innes NSW

phone 09-427 0373
fax 09-426 7331
email cato@bolam.com.au

ORIGINAL SCALE	ORIGINAL SIZE	REVISION NO
1 : 2000	A3	R1
DATE	CAD REFERENCE	SHEET NO
15/11/2016	31245 E112 CutFil.p	E112
DIRECTORY		JOB NO
2/3/16/15/16/16		34745



LF14 Rev.7 Dynamic Cone Penetration (DCP) Test Report

NZS 4402: Test 6.5.2: 1988



LF14 Rev.7 Dynamic Cone Penetration (DCP) Test Report

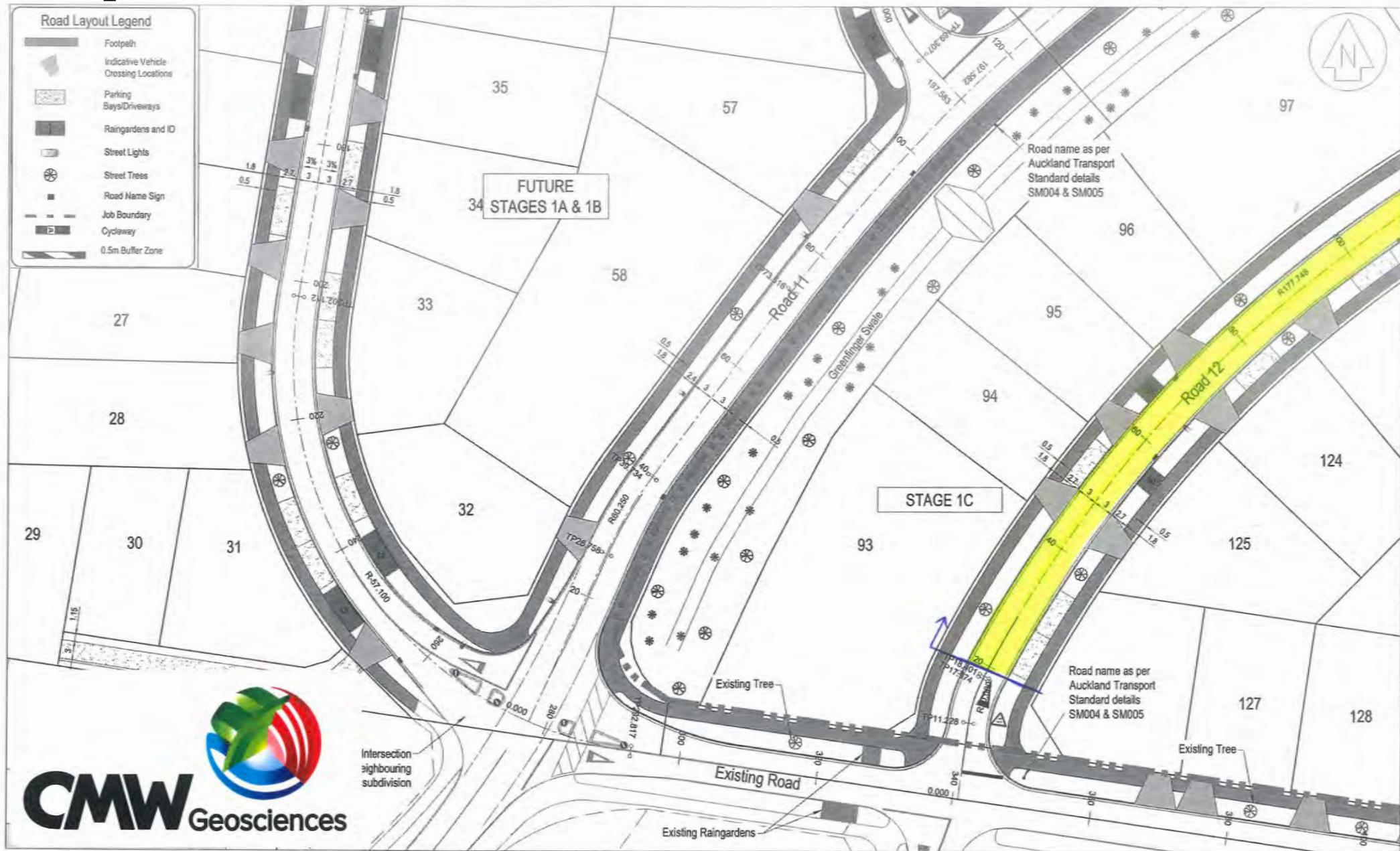
NZS 4402: Test 6.5.2: 1988

Report No:	AKL2016_0634LAC Rev.0								Auckland Laboratory CMW Geosciences (NZ) Limited Building C, 9 Piermark Drive, Rosedale, NZ 0632 PO Box 300206, Albany, Auckland, NZ 0752 Phone: +64 (09) 4144 632	
Project Name:	45 Station Road									
Project Location:	Huapal									
Project Number:	AKL2016_0634									
Test Date:	11/01/2016								Testing Locations Selected By: CMW Field Staff	
Tested By:	RHD/KP									
Client:	Cabra Developments Limited									
Client Address:										
Client Reference:										
Test No.	11		12		13		14		15	
Test Location	Road 12		Road 12		Road 12		Road 12		Road 12	
Chainage & Offset	CH120 Left		CH130 Right		CH140 Left		CH150 Right		CH160 Left	
Material & Layer:	CLAY/ Subgrade +130mm		CLAY/ Subgrade +130mm		CLAY/ Subgrade +130mm		CLAY/ Subgrade +130mm		CLAY/ Subgrade +130mm	
Depth (mm)	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*
0 - 100	8	18	2	4	3	6	4	8	4	B
100 - 200	6	13	3	6	1	2	1	2	3	6
200 - 300	3	6	3	6	1	2	1	2	4	8
300 - 400	2	4	3	6	1	2	1	2	6	13
400 - 500	2	4	4	8	1	2	2	4	6	13
500 - 600	2	4	5	10	1	2	1	2	8	18
600 - 700	2	4	5	10	2	4	1	2	8	18
700 - 800	2	4	6	13	2	4	1	2	9	20
800 - 900	2	4	5	10	2	4	2	4	9	20
900 - 1000										
Test No.	16		17		18		19		20	
Test Location	Road 12		Road 12		Road 12		Road 12		Road 12	
Chainage & Offset	CH170 Right		CH220 Left		CH230 Right		CH240 Left		CH250 Right	
Material & Layer:	CLAY/ Subgrade +130mm		CLAY/ Subgrade +130mm		CLAY/ Subgrade +130mm		CLAY/ Subgrade +130mm		CLAY/ Subgrade +130mm	
Depth	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*
0 - 100	11	20+	1	2	1	2	0	0	0	0
100 - 200	6	13	2	4	1	2	2	4	2	4
200 - 300	7	15	2	4	2	4	2	4	2	4
300 - 400	6	13	4	8	2	4	2	4	2	4
400 - 500	5	10	4	8	4	8	4	8	2	4
500 - 600	5	10	6	13	4	8	4	8	2	4
600 - 700	5	10	9	20	5	10	5	10	6	13
700 - 800	5	10	6	13	6	13	6	13	4	8
800 - 900	5	10	10	20+	6	13	6	13	4	8
900 - 1000										
This report should only be reproduced in full										
Prepared by:	CS	Date:	12/01/2017		*Equivalent CBR values calculated using Austroads (2010) Guide to Pavement Technology Part 2, Figure 5.3, For Fine Grained Cohesive Soils, and are relevant to fine grained cohesive soils only.					
Checked by:	TG	Date:	12/01/2017							
Authorised Signatory:		Date:	12/1/2017		Page 2 of 5					



LF14 Rev.7 Dynamic Cone Penetration (DCP) Test Report

NZS 4402: Test 6.5.2; 1988



CMW Geosciences

CATO BOLAM
CONSULTANTS

SURVEYORS **PLANNERS** **ENGINEERS**

CADD BOLAND CONSULTANTS LTD
19 Temeriki Avenue
PO Box 157
Stevens 0046

phone 09-
fax 09-
belen@xatxai

REVISION (DESCRIPTIONS)	NAME	DATE
SURVEYED	NAME	DATE
DESIGNED	KM	09/16
DRAWN	SL	09/16
CHECKED		
APPROVED		

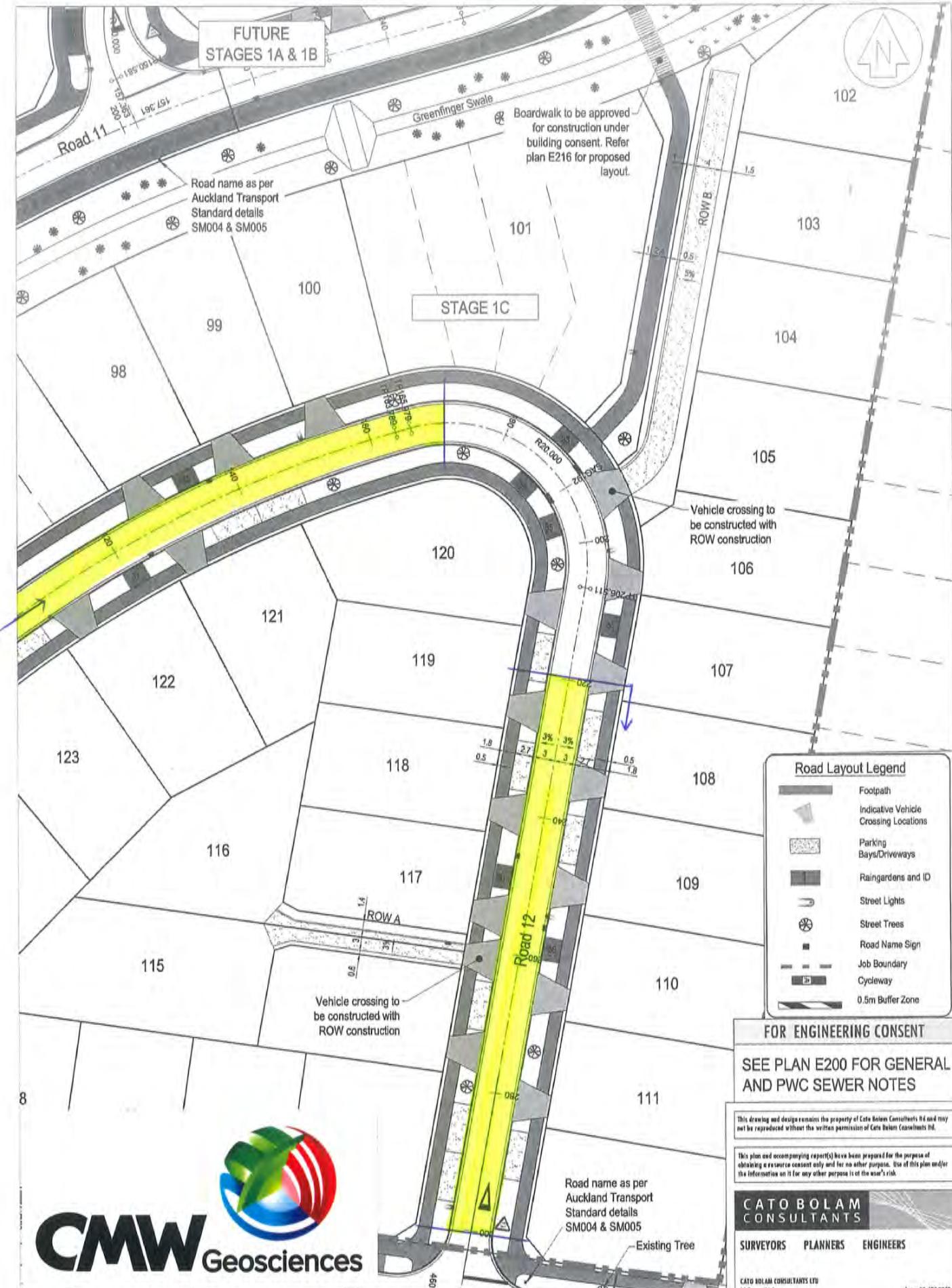
This plan and accompanying report(s) have been prepared for the purpose of obtaining a resource consent only and for no other purpose. Use of this plan and/or the information it contains for any other purpose is at the user's risk.

9

CABRA DEVELOPMENTS LTD
45 STATION ROAD,
HUAPAI

DRAWING TITLE

ORIGINAL SCALE 1:500	ORIGINAL SIZE A3	REVISION NO
DATE 5/09/2016	CAD REFERENCE 34745 E200 ROAD LAYOUT	SHEET NO E205
RECTORY	IMAGE FILE	JOB NO 34745
1:500		



REVISION (DESCRIPTIONS)	NAME	DATE
SURVEYED	NAME	DATE
DESIGNED	KM	09/16
DRAWN	SL	09/16
CHECKED		
APPROVED		

CLIENT

CABRA DEVELOPMENTS LTD
45 STATION ROAD,
HUAPAI

DRAWING TIT

ROADING LAYOUT
SHEET 7 OF 7
STAGE 1C

PO Box 157 Dovee 0944	fax 09 426 7331 nml112@xtra.co.nz	
ORIGINAL SCALE 1:500	ORIGINAL SIZE A3	REVISION NO
DATE 5/09/2016	CAD REFERENCE 34745 1200 ROAD LAYOUT	SHEET NO E206
DIRECTORY 1/2M14/1640		JOB NO 34745

LF11 Rev 4 Soil Field Density NDM Direct Transmission with VSS Report

Auckland Laboratory
CMW Geosciences (NZ) Limited
Building C, 9 Piermark Drive, Rosedale, NZ 0632
PO Box 300206, Albany, Auckland, NZ 0752
Phone: +64 (09) 4144 632

Project: 45 Station Road, Huapai
Project No: AKL2016_0634
Location: Huapai
Report No: AKL2016_0634LAA Rev.0
Report Date: 25/01/2017
Client: Cabra Developments Limited
Client Address:
Client Reference:

Test Methods: Notes:
NZS 4402.2.1:1986 Solid Density: Assumed
NZS 4407.4.2.2:2015 Testing Locations Selected By: CMW Field Staff
NZGS: August 2001



Tests indicated as
not accredited are outside
the scope of the
laboratory's accreditation

Measurements marked * are not accredited
and are outside the scope of the laboratories
accreditation

Date Sampled	Sample No.	Test Location	Soil Description	In-situ Vane Shear Strengths					Field and Laboratory Testing Data							Comments	
				Test 1 (kPa)	Test 2 (kPa)	Test 3 (kPa)	Test 4 (kPa)	Ave.	Gauge Wet Density (t/m³)	Gauge Dry Density (t/m³)	Gauge Water Content (%)	Gauge Air Voids (%)	Gauge Probe Depth	Oven Water Content (%)	Solid Density (t/m³) *	Oven Dry Density (t/m³)	
7/12/2016	N1	Lot 97	CLAY	189	189	>189	>189	>189	1.8543	1.3865	33.7	1.76	300	29.3	2.7	1.44	4.9
	N2	Lot 99	CLAY	>189	>189	>189	>189	>189	1.8482	1.3592	36.0	0.65	300	37.4	2.7	1.34	-0.1
14/12/2016	N3	Refer to site plan	CLAY	UTP	UTP	UTP	UTP	UTP	1.7574	1.3213	33.0	7.35	300	29.6	2.7	1.36	9.6
	N4	Refer to site plan	CLAY	173	178	189	189	182	1.8379	1.3691	34.2	2.31	300	35.0	2.7	1.36	1.9
16/12/2016	N5	Lot 93	CLAY	175	173	189	189	182	1.7953	1.2722	41.1	0.45	250	31.0	2.7	1.38	6.8
	N6	Lot 97	CLAY	UTP	UTP	UTP	UTP	UTP	1.7946	1.3099	37.0	2.91	300	33.4	2.7	1.34	5.2
	N7	Refer to site plan	CLAY	>189	>189	UTP	UTP	>189	1.8157	1.3169	37.9	1.24	250	30.8	2.7	1.38	5.9
	N8	Lot 86	CLAY	UTP	UTP	UTP	UTP	UTP	1.8583	1.3689	35.7	0.25	300	38.6	2.7	1.34	-1.4
20/12/2016	N9	Lot 93	CLAY	UTP	UTP	>189	>189	>189	1.8210	1.3141	38.6	0.52	300	34.9	2.7	1.34	2.9
	N10	Lot 95	CLAY	UTP	UTP	>189	>189	>189	1.8331	1.3237	38.5	-0.08	300	33.5	2.7	1.38	3.2
22/12/2016	N11	Road	CLAY	189	186	183	159	179	1.7840	1.3012	37.7	2.30	300	36.3	2.7	1.30	4.0
30/12/2016	N12	Lot 44	Silty CLAY	>194	>194	193	141	>181	1.7770	1.2843	38.4	3.05	300	48.3	2.7	1.20	-2.2 Failed
	N13	Lot 45	Clayey SILT	UTP	151	158	148	>162	1.7915	1.3219	35.5	3.98	300	38.7	2.7	1.30	2.2
	N14	Lot 47	Silty CLAY	>194	>194	>194	>194	>194	1.8021	1.3070	37.9	1.97	300	39.5	2.7	1.30	1.1
	N15	Lot 46	Silty CLAY	>194	>194	>194	>194	>194	1.7969	1.3130	36.8	2.88	300	40.3	2.7	1.28	1.0
4/01/2017	N16	Lot 62	Silty CLAY	UTP	UTP	UTP	UTP	UTP	1.8617	1.3590	37.0	-0.72	300	31.6	2.7	1.42	2.9
	N17	Lot 63	Silty CLAY	UTP	UTP	UTP	UTP	UTP	1.8246	1.3219	38.0	0.66	300	36.7	2.7	1.34	1.6
	N18	Lot 44	Silty CLAY	UTP	UTP	UTP	UTP	UTP	1.8530	1.3908	33.2	2.17	300	37.4	2.7	1.34	-0.4 Retest of N12
5/01/2017	N19	Lot 64	Silty CLAY	188	151	154	157	163	1.7956	1.2963	38.5	1.95	300	54.4	2.7	1.16	-6.3 Failed
	N20	Road	Silty CLAY	>194	>194	>194	148	>182	1.7699	1.2558	40.9	1.96	300	28.7	2.7	1.38	9.6
6/01/2017	N21	Lot 44	CLAY	UTP	UTP	UTP	UTP	UTP	1.8059	1.3199	36.8	2.44	300	44.8	2.7	1.24	-2.0
	N22	Lot 42	CLAY	>194	>194	191	>194	>193	1.8125	1.3097	38.7	1.09	300	32.9	2.7	1.36	4.6
	N23	Lot 64	CLAY	UTP	UTP	UTP	UTP	UTP	1.8148	1.3314	36.3	2.25	300	45.2	2.7	1.26	-1.7 Retest of N19
10/01/2017	N24	Lot 43	CLAY	189	189	>189	>189	>189	1.7905	1.3118	36.5	3.44	300	36.3	2.7	1.32	3.6
	N25	Lot 19	CLAY	178	189	189	>189	>186	1.7855	1.2864	38.8	2.33	300	38.5	2.7	1.28	2.6
12/01/2017	N26	Lot 14	CLAY	>189	>189	UTP	UTP	>189	1.7964	1.2794	40.4	0.80	300	41.3	2.7	1.28	0.4
	N27	Lot 17	CLAY	>189	>189	UTP	UTP	>189	1.7357	1.2292	41.2	3.71	300	36.9	2.7	1.26	6.3

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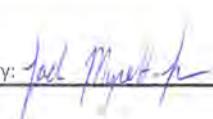
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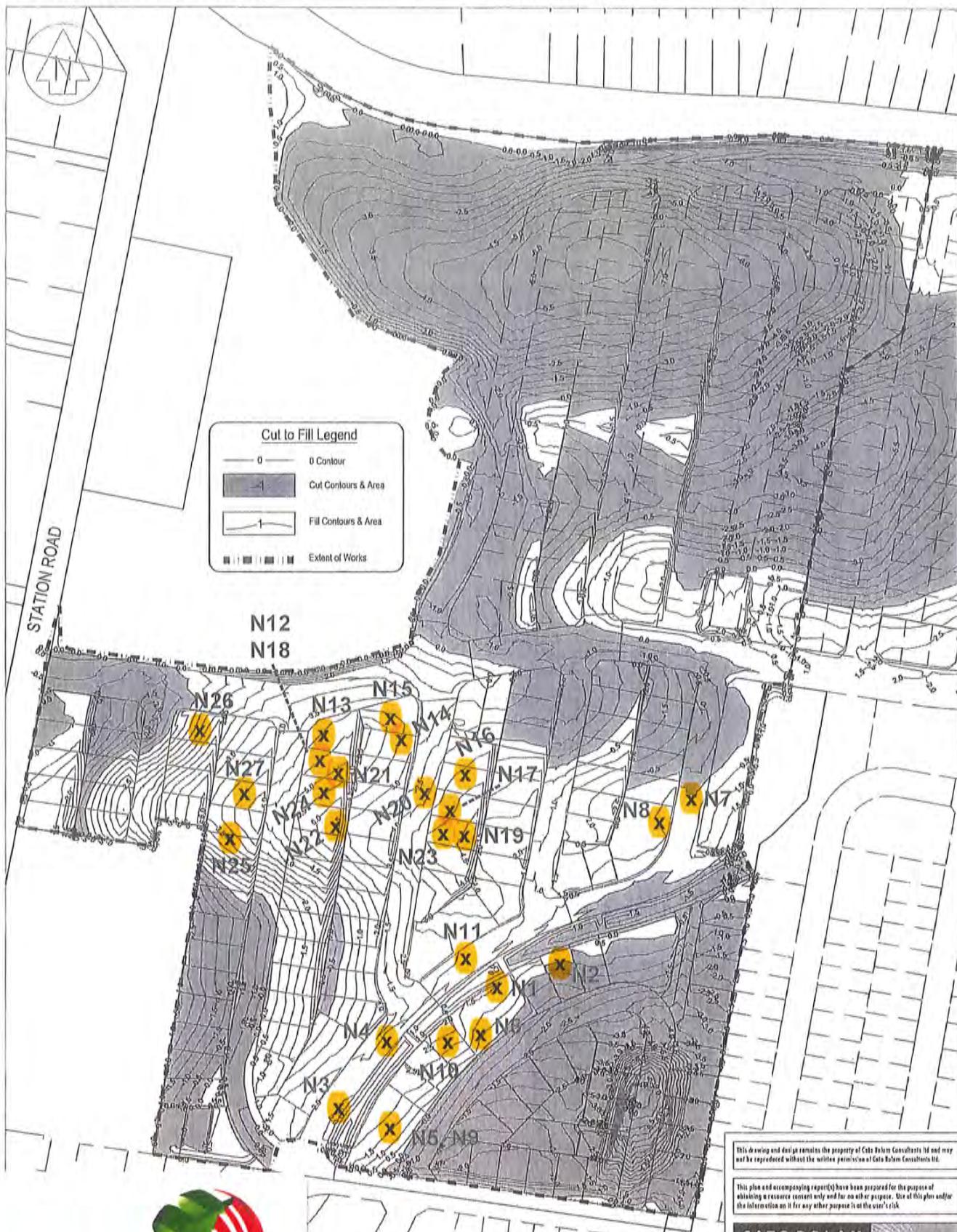
Checked By: TG

Date: 25/01/2017

Authorised Signatory:



Date: 25/01/2017



R1 REVISED FOR AMENDED DESIGN	KM	05/12/2016
REVISION (DESCRIPTION)	NAME	DATE
SURVEYOR	KM	15/11/2016
DESIGNER	KM	15/11/2016
DRAWH	KM	15/11/2016
CHECKED		
APPROVED		

CABRA DEVELOPMENTS LTD
45 STATION ROAD,
HUAPAI

DRAWING TITLE

CUT AND FILL DEPTH
CONTOURS PLAN

ORIGINAL SCALE	ORIGINAL SIZE	REVISION NO
1 : 2000	A3	R1
15/11/2016	31P45 E112 CutFil.p	E112
Z:\3D\45\45\ACAD		JOB NO 34745

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**CATO BOLAM
CONSULTANTS**

SURVEYORS PLANNERS ENGINEERS

CATO BOLAM CONSULTANTS LTD
19 Terrell Avenue
PO Box 157
Dunedin 9016

phone 03-427 0072
fax 03-434 2331
email cato.bolam@xtra.co.nz

Appendix E

Producer Statement

13 September 2017

Ref: AKL2016-0634AF Rev. 0

Cabra Developments Limited
PO Box 197, Orewa 0946
Auckland, New Zealand

Attention: Duncan Unsworth

Dear Duncan

**RE: CERTIFICATION OF SITEWORKS FOR THE CONSTRUCTION OF TIMBER POLE
RETAINING WALLS AND KEYSTONE WALLS AT 45 STATION ROAD – BCO10090856**

CMW Geosciences have visited the site at 45 Station Road, Huapai, legally described as Lot 119 DP 508664 on several occasions during January to March to observe the construction of timber pole and keystone retaining walls.

Our work has included review of the following documents and drawings:

- Conditions of Auckland Council Building Consent referenced BCO10090856, issued 23 March 2017;
- Consented construction drawings, prepared by CMW Geosciences (NZ) Limited, referenced AKL2016_0634AC Rev. 0 Figures 100 to 105 inclusive, and dated 20 January 2017;
- Geotechnical report for Huapai Development Stage 1 prepared by CMW Geosciences, referenced 2015_1029AB Rev. 0, dated 24 November 2014.

The site works observed and/or tested by CMW staff incorporated:

- Timber pole retaining walls
 - Assessment of soil strengths in the exposed pile foundations excavations;
 - Pile size, depth, spacing and diameter;
 - Timber pole size, treatment and placement;
 - Lagging dimensions and placement;
 - Drainage material and installation.
- Keystone retaining walls
 - Assessment of soil strengths in the exposed foundations excavations;
 - Foundation dimensions;
 - Geogrid reinforcement placement (type, length, vertical spacing);
 - Hardfill compaction;
 - Drainage material and installation.

Construction aspects of the timber pole and keystone retaining walls were checked for the aforementioned hold points over several visits.

Hardfill compaction of the reinforced hardfill was both checked by proof rolling on site to observe deflection under compaction plant and tested using an impact hammer to check levels of compaction.

Ground conditions were tested using a handheld shear vane at the base and sides of the pile excavation. The retained soil was also tested as part of the observations.

When water was observed in the pile excavations the contractor was advised to either tremie the concrete to the base of the excavation or remove the water immediately prior to pouring concrete. CMW were not present at the time of concrete pouring.

CMW did not observe the retaining wall drainage being connected to the reticulated stormwater system.

On the basis of our observations and testing, we consider that the site works have been undertaken in accordance with the approved Building Consent and related approved documentation described above and are in accordance with the requirements and/or recommendations of the geotechnical report.

For and on behalf of
CMW Geosciences NZ Ltd



Richard Knowles

Principal Geotechnical Engineer, CPEng

Attachments: Producer Statement - Construction Review

PRODUCER STATEMENT – PS4 – CONSTRUCTION REVIEW

(Guidance notes on the use of this form are printed on page 2)

ISSUED BY: ...CMW GEOSCIENCES.....
(Construction Review Firm)

TO: ...CABRA DEVELOPMENTS LIMITED.....
(Owner/Developer)

TO BE SUPPLIED TO: ...AUCKLAND COUNCIL.....
(Building Consent Authority)

IN RESPECT OF: ...CONSTRUCTION OF TIMBER POLED RETAINING WALLS AND KEYSTONE WALLS.....
(Description of Building Work)

AT: ...45 STATION ROAD, HUAPAI.....
(Address)
LOT.....119..... DP ...508664... SO

...CMW GEOSCIENCES.....has been engaged by...CABRA DEVELOPMENTS LIMITED.....
(Construction Review Firm)

To provide CM1 CM2 CM3 CM4 CM5 (Engineering Categories) or observation as per agreement with owner/developer

or other ...REFER TO DOCUMENT AKL2016-0634AF REV 0 DATED 13 SEPTEMBER 2017..services
(Extent of Engagement)

in respect of clause(s)B1..... of the Building Code for the building work described in
documents relating to Building Consent No. ...BCO10090856..... and those relating to
Building Consent Amendment(s) Nos. issued during the
course of the works. We have sighted these Building Consents and the conditions of attached to them.

Authorised instructions / variations(s) No. (copies attached)
or by the attached Schedule have been issued during the course of the works.

On by the basis of this these review(s) and information supplied by the contractor during the course of the works and
on behalf of the firm undertaking this Construction Review, I believe on reasonable grounds that All Part only of
the building works have been completed in accordance with the relevant requirements of the Building Consent and Building
Consent Amendments identified above, with respect to Clause(s)B1..... of the Building Code.

I also believe on reasonable grounds that the persons who have undertaken this construction review have the necessary
competency to do so.

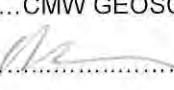
I, ...RICHARD J KNOWLES...(AC AUTHOR 2342).....am: CPEng No. ...160049.....
(Name of Construction Review Professional)
 Reg Arch No.

I am a Member of : IPENZ NZIA and hold the following qualifications: ...BE(CIVIL), CPEng.....

The Construction Review Firm issuing this statement holds a current policy of Professional Indemnity Insurance no less
than \$200,000*.

The Construction Review Firm is a member of ACENZ :

SIGNED BY ...RICHARD J KNOWLES..... ON BEHALF OF ...CMW GEOSCIENCES.....

Date:18/7/17..... Signature:

Note: This statement shall only be relied upon by the Building Consent Authority named above. Liability under this statement accrues to the
Design Firm only. The total maximum amount of damages payable arising from this statement and all other statements provided to the Building
Consent Authority in relation to this building work, whether in contract, tort or otherwise (including negligence), is limited to the sum of
\$200,000*.

This form is to accompany Forms 6 or 8 of the Building (Form) Regulations 2004 for the issue of a Code Compliance
Certificate.

13 September 2017

Ref: AKL2016_0634AG Rev. 0

Cabra Developments Limited
PO Box 197, Orewa 0946
Auckland

Attention: Duncan Unsworth

Dear Duncan

**RE: CERTIFICATION OF SITEWORKS FOR THE CONSTRUCTION OF TWO TIMBER
BOARDWALKS AT 53 STATION ROAD – BCO10091660**

CMW Geosciences (CMW) have visited the site at 53 Station Road, Huapai legally described as Lot 1 DP 502602 on several occasions during August 2017 to observe the site works for the construction of two timber boardwalks.

Our work has included review of the following documents and drawings:

- Conditions of Auckland Council Building Consent referenced BCO10091660, issued 18 April 2017;
- Consented construction drawings, prepared by Hutchinson Consulting Engineers Limited, referenced 19870 S01-S08, dated Feb 2017;
- Geotechnical report for Huapai Development Stage 1 prepared by CMW Geosciences, referenced 2015_1029AB Rev. 0, dated 24 November 2014.

The site works observed and/or tested by CMW staff incorporated:

- assessment of soil strengths in the exposed foundation excavations;
- pile size, depth and diameter.

Our testing demonstrated vane shear strengths in excess of the 50kPa design specification for alluvial soils, in excess of the 100kPa design specification for engineered fill and in excess of the 200kPa design specifications for rock. Pile holes were observed as having been imbedded a minimum of 300mm into the rock.

On the basis of our observations and testing, we consider that the site works have been undertaken in accordance with the approved Building Consent and related approved documentation described above and are in accordance with the requirements and/or recommendations of the geotechnical report.

For and on behalf of CMW Geosciences



Richard Knowles

Principal Geotechnical Engineer, CPEng

Attachments: Producer Statement - Construction Review

PRODUCER STATEMENT – PS4 – CONSTRUCTION REVIEW

(Guidance notes on the use of this form are printed on page 2)

ISSUED BY: ...CMW GEOSCIENCES.....
(Construction Review Firm)

TO: ...CABRA DEVELOPMENTS LIMITED.....
(Owner/Developer)

TO BE SUPPLIED TO: ...AUCKLAND COUNCIL.....
(Building Consent Authority)

IN RESPECT OF: ...SITWORKS FOR THE CONSTRUCTION OF TWO TIMBER BOARDWALKS.....
(Description of Building Work)

AT: ...53 STATION ROAD, HUAPAI.....
(Address)
.....LOT.....1..... DP ...502602... SO

...CMW GEOSCIENCES.....has been engaged by...CABRA DEVELOPMENTS LIMITED.....
(Construction Review Firm)

To provide CM1 CM2 CM3 CM4 CM5 (Engineering Categories) or observation as per agreement with owner/developer or other ...REFER TO ATTACHED DOCUMENT AKL2016_0634AG REV. 0 DATED 13 SEPTEMBER 2017..services in respect of clause(s)B1..... of the Building Code for the building work described in documents relating to Building Consent No. ...BCO10091660..... and those relating to Building Consent Amendment(s) Nos.issued during the course of the works. We have sighted these Building Consents and the conditions of attached to them.

Authorised instructions / variations(s) No. (copies attached) or by the attached Schedule have been issued during the course of the works.

On by the basis of this these review(s) and information supplied by the contractor during the course of the works and **on behalf of the firm** undertaking this Construction Review, I believe **on reasonable grounds** that All Part only of the building works have been completed in accordance with the relevant requirements of the Building Consent and Building Consent Amendments identified above, with respect to Clause(s)B1..... of the Building Code. I also believe on reasonable grounds that the persons who have undertaken this construction review have the necessary competency to do so.

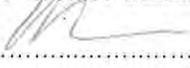
I, ...RICHARD J KNOWLES (AC AUTHOR #2342)..... am: CPEng No. ...160049.....
(Name of Construction Review Professional) Reg Arch No.

I am a Member of : IPENZ NZIA and hold the following qualifications: ...BE(CIVIL), CPEng.....

The Construction Review Firm issuing this statement holds a current policy of Professional Indemnity Insurance no less than \$200,000*.

The Construction Review Firm is a member of ACENZ :

SIGNED BY ...RICHARD J KNOWLES..... ON BEHALF OF ...CMW GEOSCIENCES.....

Date: ... 18/9/17 Signature: 

Note: This statement shall only be relied upon by the Building Consent Authority named above. Liability under this statement accrues to the Design Firm only. The total maximum amount of damages payable arising from this statement and all other statements provided to the Building Consent Authority in relation to this building work, whether in contract, tort or otherwise (including negligence), is limited to the sum of \$200,000.*

This form is to accompany Forms 6 or 8 of the Building (Form) Regulations 2004 for the issue of a Code Compliance Certificate.