

25 June 2019

236 Matua Road Huapai

GEOTECHNICAL COMPLETION REPORT

Cabra Developments Limited AKL2016_0630AH Rev 0

AKL2016_0630AH					
Date Revision		Comments			
18 June 2019	Α	Initial draft for internal review			
24 June 2019	В	Final draft for internal review			
25 June 2019	0	Final issue to client			

	Name	Signature	Position
Prepared by	Jack Mynett – Johnson	1 Myreth-Jehnson	Project Engineering Geologist
Reviewed by	Andrew Linton		Principal Geotechnical Engineer
Authorised by	Richard Knowles	RJ Knowles	Principal Geotechnical Engineer, CPEng







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1. INTRODUCTION

In accordance with our instructions, this Geotechnical Completion Report has been prepared for Cabra Developments Limited as part of the documentation to be submitted to Auckland Council following earthworks to form the 236 Matua Road development. Construction of this residential subdivision has been undertaken in accordance with the Auckland Council Resource Consent numbers BUN60309658, SUB60309875, LUC60309876, and DIS60316310 and Engineering Approval letter dated 25 October 2018. Specific structures constructed during the civil works to create the subdivision include timber pole retaining walls and segmental block retaining walls.

This report contains our Suitability Statement, specific comments related to items raised in the Resource Consent, relevant test data and the Aspire Consulting Engineers Limited as-built plan set as provided in Appendix B.

This report covers the construction period October 2018 to May 2019 and is intended to be used for certification purposes for new lots (listed below) created from Lot 3 DP 171599 as follows:

- 19 new residential lots numbered lots 1 to 19:
- 1 Common Access Lot numbered lot 20;
- 1 extension of the existing Vogwill Road, numbered lot 21;
- 2 Local Purpose Esplanade Reserves numbered lots 22 and 23;
- 2 Local Purpose Accessways numbered lots 24 and 25; and,

The 236 Matua Road Development is located off Vogwill Road, Huapai. As can be seen from the as-built plans, 10 of the lots have been affected by filling as part of the earthworks operations to a maximum depth of approximately 3 metres.

2. PROJECT BACKGROUND

The geotechnical investigations and retaining wall designs were undertaken by CMW Geosciences as presented in the following reports:

- CMW Geosciences Geotechnical Investigation Report for a Proposed Residential Subdivision at 236 Matua Road, Huapai referenced AKL2016_0630AA Rev. 0 dated 31 January 2017;
- CMW Geosciences Timber Retaining Wall Design Report for Lots 18 and 19, 236 Matua Road, Huapai referenced AKL2016-0630AB Rev. 0 dated 30 July 2018;
- CMW Geosciences Eastern Keystone Retaining Wall Design Report for 236 Matua Road, Huapai referenced AKL2016_0630AC Rev. 1 dated 8 October 2018;
- CMW Geosciences Western Keystone Retaining Wall Design Report for 236 Matua Road, Huapai referenced AKL2016_0630AD Rev. 1 dated 8 October 2018; and,
- CMW Geosciences Geotechnical Review of Earthworks Plans for 236 Matua Road, Huapai referenced AKL2016 0630AE Rev. 0 dated 20 August 2018.

3. DESCRIPTION OF EARTHWORKS

Works within the site began in early October 2018 with the removal of existing buildings, accessways, fences and vegetation from the earthworks area by the main contractor Opie Contractors Limited. Silt and erosion control measures were also constructed during this time by earthworks subcontractor Bob Hick Earthmoving

Limited. This including the construction of two temporary silt detention ponds, one within Lots 6 and 7, the second in the lower portion of Lot 13.

Topsoil stripping operations across much of the site began in mid-October with topsoil stockpiled within Lots 9, 10 and 11. Soft alluvial materials were removed from the two gullies within Lots 4 and 5 and Lots 7, 8 and 9, and spread out to be dried before being blended in with other fill materials. Underfill subsoil drains consisting of geotextile cloth wrapped scoria filled drainage trenches with a single 160mm perforated drain coil were installed at the base of the gullies.

Following the installation of the subsoil drainage, cut and fill earthworks operations began throughout the site. Earthworks were mostly complete by early December 2018.

Civil operations began in late November 2018 including construction of the timber pole and segmental block retaining walls, construction of stormwater and sanitary sewer service lines, and road pavement construction.

In January 2019, topsoil began to be respread across the site and the temporary silt pond in Lot 13 was disestablished and removed.

From late January to May 2019 civil works continued including the completion of the retaining walls, combined services, kerb and channel formation, rain garden installation, pavement formation, street lighting and footpaths.

In May 2019 the temporary silt detention pond was decommissioned and backfilled with engineered fill.

4. GEOTECHNICAL QUALITY CONTROL

4.1. Site Observations

During the earthworks site visits were typically undertaken several times each week to assess compliance with NZS 4431 and specific design recommendations and specifications.

Site visits were carried out to observe and confirm compliance relating to:

- Adequate topsoil stripping;
- Fill areas prior to the placement of fill materials to ascertain that all mullock and soft inorganic subsoils had been removed;
- Installation of subsoil drains including underfill drains but excluding road under-channel drains;
- Backfilling of subsoil drains;
- Excavation and backfilling of sewer and stormwater trenches;
- Construction of cantilever pole retaining walls including ground conditions, pile size, spacing and depth, and drainage installation;
- Construction of segmental block retaining walls including ground conditions, block placement, nofines width and drainage installation: and,
- Placement and compaction of engineered fills.

4.2. Compaction Control

Compaction of engineered earth fills was controlled by undrained shear strength measured by hand held shear vane calibrated using the NZGS 2001 method and by air voids as defined by NZS4402.

The criteria for undrained shear strength were a minimum single value of 110 kPa and minimum average of any 10 consecutive tests of 140 kPa.

The criteria for air voids were a maximum single value of 12% and maximum average of any 10 consecutive tests of 10%.

Vane shear strength, water content and in situ density tests were carried out on all areas of the engineered filling to at least the frequency recommended by NZS 4431.

These tests showed on multiple occasions that the required compaction standards were not being achieved and to the best of our knowledge the failing areas of fill were re-worked as necessary. Subsequent testing confirmed compliance with the specification.

5. EVALUATION OF COMPLETED EARTHWORKS

5.1. Natural Hazards

The appended as-built drawings depict the extents of a series of zones that contain limitations intended to ensure that future building and/ or earthworks on the lots is undertaken in a manner that does not lead to buildings being subject to any of the natural hazards described in Section 71(3) of the Building Act, i.e. erosion, falling debris, subsidence, slippage, and inundation. Consideration of the inundation hazard was outside the scope of CMW's brief and has been assessed by others. The applied zones include:

- Flood Plain Consent Notice Zone Lots 13 and 14 are situated partially within flood plains from the adjacent Kumeu River. We understand that the flood plain zone within these lots is at an RL of 16.3m (including allowance for climate change). No building development and /or future earthworks should be undertaken below this level.
- Specific Design Zones (Retaining) intended to protect the retaining walls from overloading at the crest or undermining at the toe that could lead to instability;
- Specific Design Zones (Slope) intended to protect building development from long term creep effects on or adjacent to steep slopes and to protect the slopes from inappropriate loading or undermining.

Full descriptions of the restrictions associated with each of these zones are presented in the Suitability Statement (Appendix A). Additional information is also provided in some of the following sections.

5.2. Land Stability and Erosion Control

Stability conditions for finished ground profiles have been assessed under a range of groundwater conditions which satisfy ultimate limit state design criteria. The soil parameters for the analyses were selected from extensive investigation undertaken at the site and from experience in this terrain. We consider that the stability results are satisfactory for all building platform areas and we are therefore satisfied that these areas are <u>not</u> subject to the natural stability hazards described in the Building Act.

On all steep land, including on engineered batter slopes, surface stability can be compromised by indiscriminate disposal of stormwater onto the ground surface and/ or by removal of vegetation.

Building and landscape designers must ensure that all runoff from solid surfaces is directed into the stormwater system. It is also important that care is paid to the disposal of stormwater during construction so that concentrated discharges (e.g. from unconnected spouting) are not directed towards steep ground.

Depths of mulch and topsoil applied to sloping areas should be limited to less than 150mm to minimise the risks of saturation leading to localised slumping on batter face. Wherever practical on such land, and particularly on steep batters, existing vegetation and grass cover should be well maintained. Any vegetation cleared beyond the immediate area of building platforms for temporary construction purposes should be replanted or replaced as soon as possible. The roots of an established vegetation cover can serve to bind the surface soils while the foliage can reduce rain infiltration and soil saturation, resulting in better resistance to erosion and shallow slumping.

5.3. Retaining Walls

Cantilever pole and segmental block retaining walls have been constructed in the locations shown on the appended Aspire Consulting Engineers as-built plan. These walls reach a maximum height of approximately 1.8 metres and were designed by CMW Geosciences Limited and the construction was observed by this consultancy. Copies of the Producer Statements - Construction Review are provided in Appendix E.

Descriptions of the building and earthworks restrictions within the vicinity of these walls (Specific Design Zones – Retaining) are contained in the Suitability Statement in Appendix A. Lots containing these zones include lots 1 to 5 inclusive, 18 and 19.

5.4. Fill Induced Settlement

On the basis of the relatively minor magnitude of fill depths on this site, together with the elapsed time since it was placed, we consider that remaining post-construction settlements will be within code limits.

5.5. Service Line Trenches

As part of the civil works, sanitary sewer and stormwater services were trenched throughout the development as shown on the appended Aspire Consulting Engineers Stormwater and Sanitary Sewer Asbuilt Plans.

Stormwater and sanitary sewer trenches in key locations contain a punched draincoil to facilitate draining of any groundwater seepages within the trench bedding. These draincoils are connected to the downstream stormwater manhole to outlet any water. This drainage has been installed as a precautionary measure that is not considered to be necessary for private connections.

As is normal on all subdivisions, building developments involving foundations within a 45 degree zone of influence from pipe inverts will require engineering input. The Auckland Council drawing referenced SW22 provided in Appendix B, extracted from Chapter 4 of the Auckland Council Code of Practice for Land development and Subdivision, depicts their requirements for stormwater pipes. Details for water and wastewater pipes are available in the Watercare COP1 - General Requirements and Procedures. The majority of lots are known to have service trenches within the lots as shown on the appended stormwater and wastewater as-built plans. The resulting restrictions are presented in the Suitability Statement below.

5.6. Subsoil Drains

The appended Aspire Consulting Engineers as-built plan shows the positions of counterfort drains which were constructed in the natural ground during the earthworks operations. The drains were installed to help control groundwater levels and extend to formed outlets within bush areas. The ongoing operation of these drains is important to the overall stability conditions of the site.

Typical trench excavations were 1 metre deep in the natural ground beneath the filling. Accordingly they are considered to be beyond the depths of anticipated foundations.

Descriptions of the restrictions are contained in the appended Suitability Statement.

5.7. Road Subgrades

Penetration resistance testing was carried out on the road subgrades during construction and the results of this testing were forwarded to Aspire Consulting Engineers Limited for pavement remedial design. Where soft ground with low equivalent CBR values was identified it was generally undercut and replaced with hardfill. All road subgrade areas were subsequently lime/ cement stabilised to achieve appropriate CBR values.

Benkelman Beam testing of the base course was carried out by Roadtest Limited on each road and those results were also forwarded to Aspire Consulting Engineers Limited.

5.8. Design of Shallow Foundations

5.8.1. Bearing Capacity

Once bulk earthworks and top-soiling of the building platforms had been completed, our staff drilled hand auger boreholes on platforms in natural ground to determine representative finished ground conditions and hence evaluate likely foundation options for future building development. Our assessments of bearing capacity for the design of shallow foundations on each building platform are contained in the appended Suitability Statement.

At current subgrade levels all lots have been assessed as having a geotechnical ultimate bearing capacity of 300 kPa within the influence of conventional shallow residential building foundation loads.

If higher geotechnical ultimate bearing capacities are required, further specific site investigation and design of foundations should be carried out prior to Building Consent application.

5.8.2. Foundation Settlements

At the bearing pressures specified above and subject to the design requirements for soil expansiveness provided below, differential settlement of shallow foundations for buildings designed in accordance with NZS 3604 (including the 600mm subfloor fill depth limit) should be within code limits.

5.8.3. Soil Expansiveness Classification

Four sets of soil tests were carried out on samples taken from likely foundation level on lots within the development.

Testing was carried out in accordance with NZS 4402, "Methods of Testing Soils for Civil Engineering Purposes" test 2.2 and 2.6 and were used in conjunction with visual-tactile assessment of the site soils to determine expansive site Classes as defined in AS 2870, "Residential Slabs and Footings – Construction". All test results are appended.

On this basis we have assessed the AS 2870 Site Class for all lots in this development to be M (moderate). Details of foundation options for this Class are contained in the appended Suitability Statement.

In recent years in Auckland, there have been examples of concrete floors and / or foundations that have been poured on dry, desiccated subgrades in summer months on expansive soils and have undergone heaving and cracking once the soil moisture contents have returned to higher levels. Foundation contractors need to be made aware of this issue and the need to maintain appropriate moisture contents in the footings and building platform subgrade between the time of excavation and the pouring of concrete.

Remedial actions that may be appropriate include platform protection with a hard fill layer, pouring of a blinding layer of concrete in footing bases and soaking of the building platform with sprinklers for an extended period.

Home owners need to be aware that the planting of high water demand plants where their roots may extend close to footings can also cause settlement damage.

5.9. Topsoil Depths

Topsoil depths have been checked by the drilling of a borehole in the approximate centre of the building platform on each lot. The results are considered indicative for each lot but may be subject to variations. Topsoil depths are between 50 and 300mm on this development.

Site specific findings are contained in the appended Suitability Statement Summary (Appendix A). However, it is possible that further levelling works have been undertaken since our investigations and accordingly, we strongly recommend that lot purchasers complete their own checks of topsoil depths.

6. CLOSURE

The appended Statement of Professional Opinion is provided to the Auckland Council and Cabra Developments Limited for their purposes alone on the express condition that it will not be relied upon by any other person. It is important that prospective purchasers satisfy themselves as to any specific conditions pertaining to their particular land interest.

Although regular site visits have been undertaken for observation, for providing guidance and instruction and for testing purposes, the geotechnical services scope did not include full time site presence. To this end, our appended Suitability Statement also relies on the Contractors' work practices and assumes that when we have not been present to observe the work, it has been completed to high standards and in accordance with the drawings, instructions and consent conditions provided to them.

Similarly it assumes that all as-built information and other details provided to the Client and/or CMW by other members of the project team are accurate and correct in all respects.

Appendix A: Statement of Professional Opinion as to the Suitability of Land for Building Development

STATEMENT OF PROFESSIONAL OPINION AS TO THE SUITABILITY OF LAND FOR BUILDING DEVELOPMENT

- I, Richard Knowles, of CMW Geosciences (NZ) Limited Partnership, Auckland, hereby confirm that:
- As a Chartered Professional Engineer experienced in the field of geotechnical engineering, I am a Geo-professional as defined in section 1.2.2 of NZS 4404 and was retained by the Developer as the Geotechnical Engineer on the 236 Matua Road, Huapai Development.
- The extent of preliminary investigations carried out to date are described in the CMW Geosciences Geotechnical Investigation Report referenced AKL2016_0630AA Rev. 0, dated 31 January 2017. The conclusions and recommendations of this document has been re-evaluated in the preparation of this report. The results of all tests carried out are also appended.
- 3. In my professional opinion, not to be construed as a guarantee, I consider that:
 - (a) The earth fills shown on the appended Aspire Consulting Engineers Limited As-built Plan have been placed in compliance with NZS 4431, the Auckland Council Unitary Plans, Auckland Council Code of Practice for Land Development and Subdivision and related documents.
 - (b) The completed earthworks give due regard to land slope and foundation stability considerations on the building platform areas, but as shown on the appended building restriction zones plans, areas on some lots have gradients steeper than 1(v) in 4 (h) (and generally up to 1(v) in 3(h)) or are adjacent to land having such gradients. Accordingly, **Specific Design Zone (Slope) areas** have been applied on Lots 11 to 15 inclusive. No building construction and no earthworks (i.e. cut or fills of any depth) should take place within the designated **Specific Design Zone (Slope) areas** unless endorsed by a Chartered Professional Engineer experienced in geomechanics and familiar with the contents of this report. The endorsement will need to consider the implications of the proposals on both global stability conditions and soil creep on the building buildings, the interaction with service pipes and associated trench backfills, control of surface water, construction sequencing, timing and temporary support requirements construction of all earthworks, foundations and retaining walls and if necessary, comment on what aspects require engineering inspections and certification.

This limitation also applies to long term landscaping works, including any proposed minor cuts either on or near batter toes to be retained by new landscaping walls that might not normally require engineering, and to landscaping fills on or immediately above the batter slopes.

- (c) Specific Design Zone (Retaining) areas have been applied on Lots 1 to 5 inclusive, 18 and 19 for the protection of the function of the retaining walls. The retaining walls on this stage of the development were designed for a maximum of 5 kPa surcharge load and 0° toe slope. No building construction and no earthworks (i.e. cut or fills) should take place that exceed these design limits on the walls unless endorsed by a Chartered Professional Engineer experienced in geomechanics and familiar with the contents of this report who consider the stability implications of the earthworks and/ or building proposals on the retaining walls.
- (d) Lots 13 and 14 are situated partially within flood plains from the adjacent Kumeu River, as shown on the appended Aspire Consulting Engineers As Built Plan referenced 1032-AB-PG105. No building development and /or future earthworks should be undertaken below this level.

CMW Geosciences Ref. AKL2016_0630AH Rev 0 Geotechnical Completion Report

- (e) The function of the subsoil drains installed beneath Lots 4 and 8 inclusive must not be impaired by any building development or landscaping works. Any bored or driven piles must be positioned to avoid damaging the draincoils. Where any subsoil drain is intercepted by building works, it must be reinstated under the direction of a Chartered Professional Engineer to ensure the integrity of the subsoil drainage system.
- (f) A geotechnical ultimate bearing capacity of 300 kPa may be assumed for shallow foundation design on the building platforms of all lots.
 - If for any reason higher geotechnical bearing capacities are required, further specific site investigation and design of foundations should be carried out prior to Building Consent application.
- (g) The expansive site Class for all lots has been assessed as AS2870 Class M (Moderate). We recommend that building designers note on the Building Consent drawings the need to maintain appropriate moisture levels across building subgrades and in footing excavations (as described in Section 5.9.3 of the Geotechnical Completion Report) for reference by foundation contractors.
- (h) The backfilling and compaction of the storm water and sanitary sewer trenches on this subdivision has been carried out to appropriate standards having regard for the prevailing ground conditions and associated compaction induced pipe loadings.
 - However, no building development should take place within the 45 degree zone of influence of drain inverts unless endorsed by specific design and by construction inspections undertaken by a Chartered Professional Engineer experienced in geomechanics to ensure that lateral stability and differential settlement issues are addressed and that building loads are transferred beyond the influence of the pipe and trench backfill. A copy of drawing SW22 extracted from Chapter 4 of the Auckland Council Code of Practice for Land development and Subdivision this document is provided in Appendix B for clarification. Details for water and wastewater pipes are available in the Watercare COP1 General Requirements and Procedures.
- (i) Subject to the geotechnical limitations, restrictions and recommendations contained in clauses 3(a), 3(b), 3(c), 3(d), 3(e) 3(f), 3(g) and 3(h) above:
 - (i) The filled and natural ground is generally suitable for residential buildings constructed in accordance with NZS 3604 and the requirements of AS2870 for the appropriate expansive soil class.
 - (ii) Where shallow foundations are appropriate, design may be carried out in accordance with AS 2870 (Class M) or alternately, a specific foundation and structural design may be undertaken by a Chartered Professional Engineer.
- 4. Road subgrades have been formed with appropriate regard for slope stability and settlement risks.

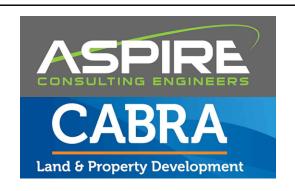
The following table summarises the conditions on each of each residential lots.

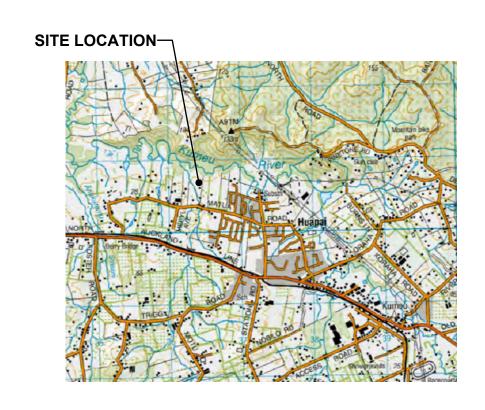
	GCR SUMMARY TABLE								
Condition	Specific Design Zone (slope)	Specific Design Zone (retaining)	Flood Level Consent Notice Zone	Subsoil Drains Present	Geotechnical Ultimate Bearing Capacity (kPa)	AS2870 Expansive Class	Service Lines Restrictions	Indicative Topsoil Depth (mm)	
GCR SOPO Clause	3(b)	3(c)	3(d)	3(e)	3(f)	3(g)	3(h)		
Lot number		1					 		
1		•			300	М		150	
2		•			300	М	•	150	
3		•			300	М	•	100	
4		•		•	300	М	•	150	
5		•			300	М	•	150	
6					300	М	•	200	
7					300	М	•	100	
8				•	300	М	•	150	
9					300	М	•	100	
10					300	М	•	150	
11	•				300	М	•	250	
12	•				300	М	•	200	
13	•		•		300	М	•	200	
14	•		•		300	М	•	150	
15	•				300	М	•	200	
16					300	М		150	
17					300	М		100	
18		•			300	М	•	150	
19		•			300	М	•	100	

Appendix B: Drawings

Title	Reference No.	Date	Revision
Aspire Consulting Engineers Preliminary and General As Built Plans	1302-AB-PG101-107	June 2019	-
Aspire Consulting Engineers Earthworks As Built Plans	1302-AB-EW201-203	June 2019	-
Aspire Consulting Engineers Roading As Built Plans	1302-AB-RD301-303	June 2019	-
Aspire Consulting Engineers Stormwater As Built Plans	1302-AB-SW401-403	June 2019	-
Aspire Consulting Engineers Wastewater As Built Plans	1302-AB-WW501	June 2019	-
Aspire Consulting Engineers Water Supply As Built Plans	1302-AB-WS601	June 2019	-
Auckland Council Stormwater Pipe and Manhole Construction Clearance Requirements	ACSD SW22	September 2013	1

CABRA DEVELOPMENTS LIMITED AS BUILT 19-LOT RESIDENTIAL SUBDIVISION AT 236 MATUA ROAD, HUAPAI BUN60306658 / ENG60324072







COVER SHEET PG101 JOB NUMBER: 1302

AS BUILT DRAWINGS:

PRELIMINARY & GENERAL WASTEWATER

PG101 COVER SHEET WW501 AS BUILT WASTEWATER PLAN

PG102 CONTENTS PAGE

PG103 EXISTING SITE PLAN WATER SUPPLY

PG104 AS BUILT SERVICE LINE RESTRICTIONS WS601 AS BUILT WATER SUPPLY PLAN

CONSENT NOTICE PLAN

PG105 AS BUILT FLOOD LEVEL CONSENT NOTICE

PLAN

PG106 AS BUILT RETAINING WALL & SLOPE SPECIFIC

DESIGN ZONE CONSENT NOTICE PLAN

PG107 AS BUILT COORDINATE TABLES

EARTHWORKS

EW201 AS BUILT CONTOUR PLAN

EW202 AS BUILT CUT & FILL DEPTH CONTOUR PLAN

EW203 AS BUILT UNDERCUT DEPTH CONTOUR PLAN

ROADING

RD301 AS BUILT ROADING PLAN

RD302-303 AS BUILT ROAD CROSS SECTIONS

STORMWATER

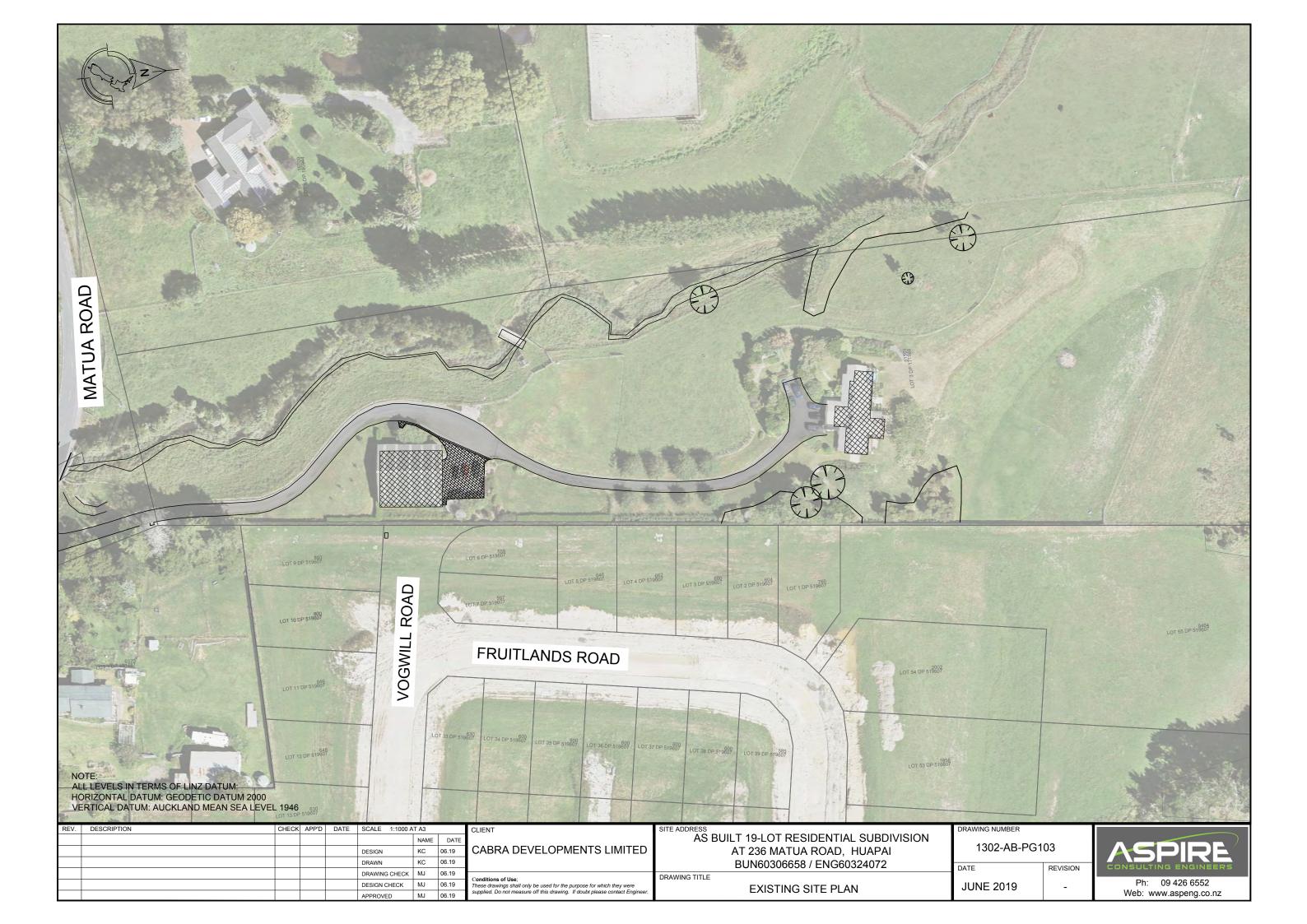
SW401 AS BUILT STORMWATER MANHOLE

AND CESSPIT PLAN

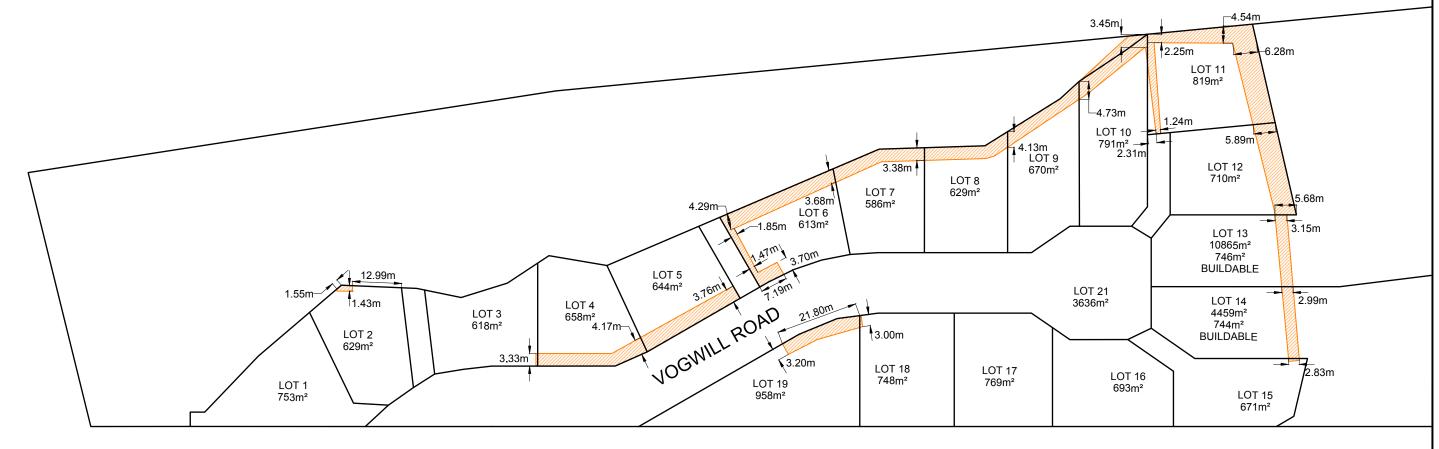
SW402 AS BUILT STORMWATER PIPE PLAN

SW403 AS BUILT RAINGARDEN DETAILS

REV.	DESCRIPTION	CHECK APP'D DAT	SCALE N.T.S.	SCALE N.T.S. AT A3 CL!		CLIENT	SITE ADDRESS	DRAWING NUMBER		
			DESIGN	NAME KC	06.19	CABRA DEVELOPMENTS LIMITED	AS BUILT 19-LOT RESIDENTIAL SUBDIVISION AT 236 MATUA ROAD, HUAPAI	1302-AB-PG1	02	ACDIDE
			DRAWN	КС	06.19		BLIN60306658 / ENG60324072	DATE	REVISION	CONSULTING ENGINEERS
			DRAWING CHECK DESIGN CHECK	MJ		Conditions of Use; These drawings shall only be used for the purpose for which they were	DRAWING TITLE CONTENTS PAGE	JUNE 2019	_	Ph: 09 426 6552
			APPROVED	MJ	06.19	supplied. Do not measure off this drawing. If doubt please contact Engineer.	CONTENTS PAGE	00.12 20.0		Web: www.aspeng.co.nz







LEGEND

SERVICE LINE RESTRICTIONS CONSENT NOTICE ZONE

DRAINAGE CONSENT NOTICE ZONE ONLY SHOWN OUTSIDE OF YARD BOUNDARIES: 3m FRONT YARD, 1m SIDE & REAR YARDS

All survey data has been collected and provided to Aspire by C&R Surveyors LTD, Registered Surveyors

I certify that these As-built Plans are an accurate record of the works undertaken and that:

- The Coordinates (X, Y) are in terms of NZTM on NZGD (2000), and are within \pm 50mm.
- The Levels (Z) are in terms of the Auckland 1946 (MSL) LINZ datum (DOSLI datum), and are within ± 10mm.

Signed: Chartered Professional Engineer

Date: 18 June 2019

Name: Evan Alexander Peters Contact Phone: (09) 426 6552 Email: evan@aspeng.co.nz

ALL LEVELS IN TERMS OF LINZ DATUM: HORIZONTAL DATUM: GEODETIC DATUM 2000 VERTICAL DATUM: AUCKLAND MEAN SEA LEVEL 1946

REV.	DESCRIPTION	CHECK	APP'D	DATE	SCALE 1:1000 A		C	
						NAME	DATE	
					DESIGN	KC	06.19	
					DRAWN	KC	06.19	
					DRAWING CHECK	MJ	06.19	
					DESIGN CHECK	MJ	06.19	C T/
					APPROVED	MJ	06.19	SL

CABRA DEVELOPMENTS LIMITED

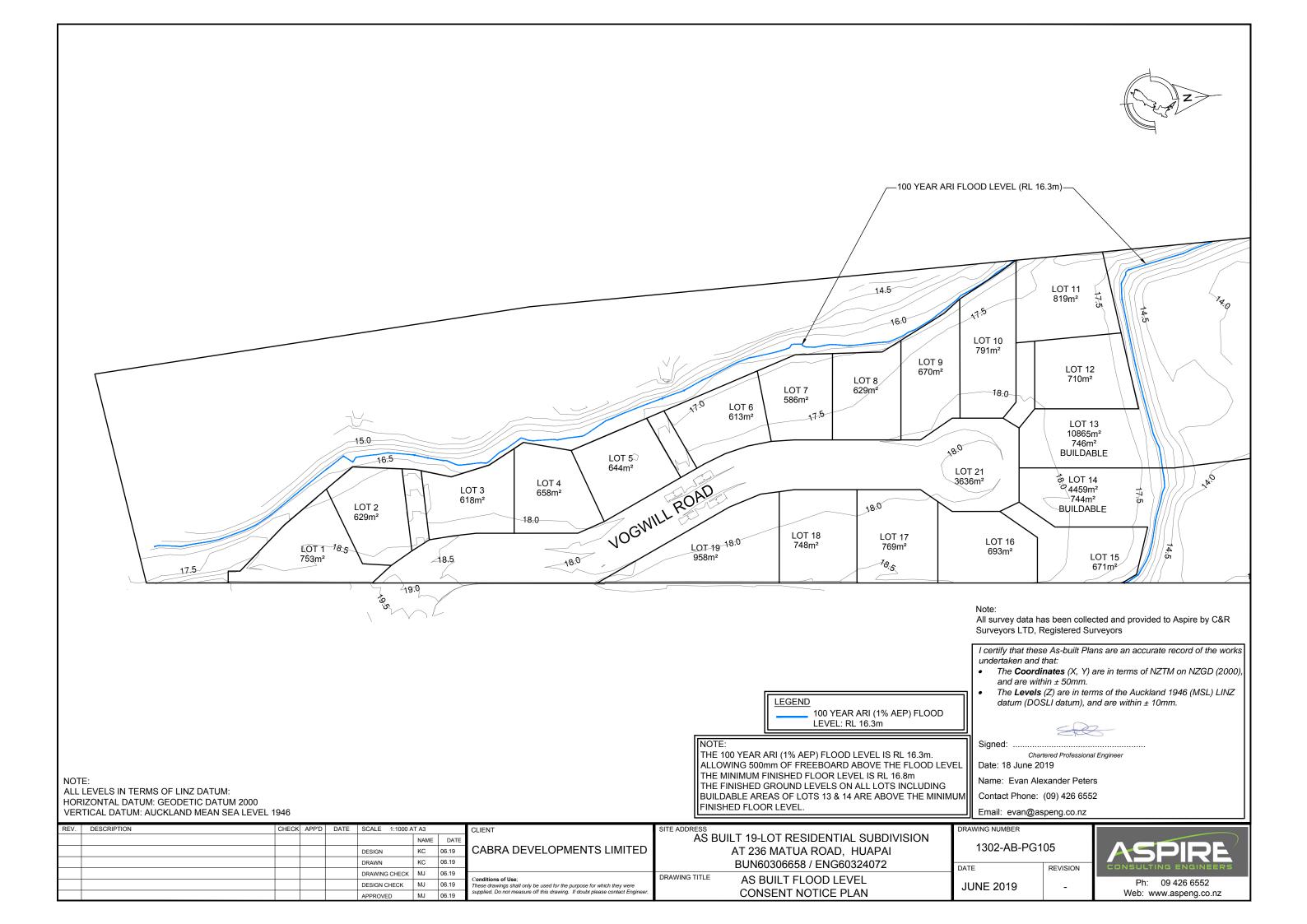
DRAWING TITLE Conditions of Use; These drawings shall only be used for the purpose for which they were supplied. Do not measure off this drawing. If doubt please contact Enginee AS BUILT SERIVCE LINE RESTRICTIONS CONSENT NOTICE PLAN

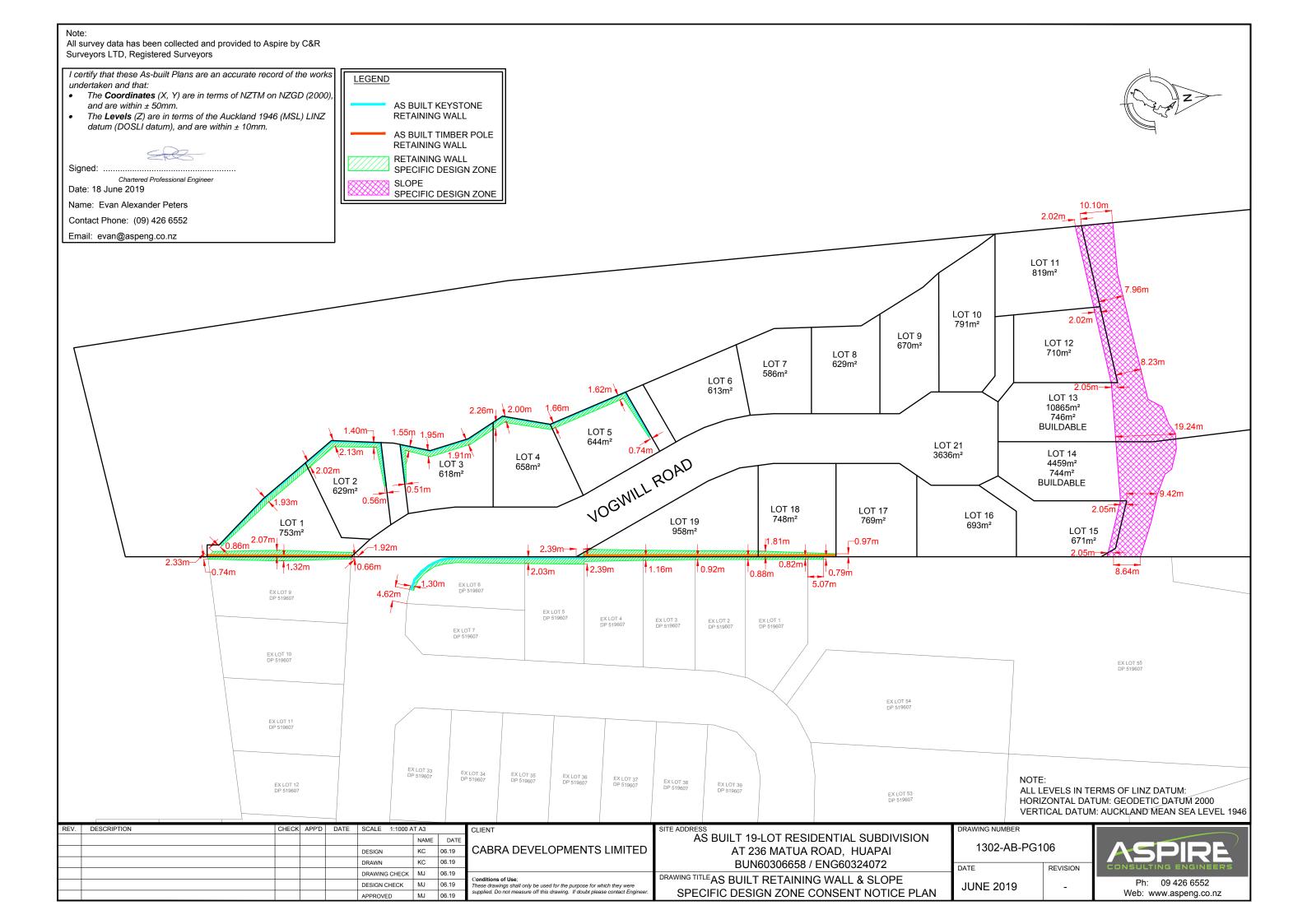
AS BUILT 19-LOT RESIDENTIAL SUBDIVISION AT 236 MATUA ROAD, HUAPAI BUN60306658 / ENG60324072

1302-AB-PG104

DATE REVISION **JUNE 2019**







AS BUILT CO-ORDINATE TABLES

STORMWATER CESSPIT

COORDINATES POINTS EASTING NORTHING AB-SWCP-DOUBLE01 1736433.445 5930567.217 AB-SWCP-DOUBLE02 1736439.689 5930569.959 AB-SWCP-SINGLE03 1736399.537 5930679.997

STORMWATER RAINGARDEN

COORDINATES								
POINTS	EASTING	NORTHING						
AB-SW-RAINGARDEN01	1736433.555	5930562.02						
AB-SW-RAINGARDEN02	1736443.343	5930566.659						
AB-SW-RAINGARDEN03	1736439.166	5930576.267						
AB-SW-RAINGARDEN04	1736429.285	5930571.9						

STORMWATER MANHOLE

COORDINATES							
POINTS	EASTING	NORTHING					
AB-SWMH-A01	1736398.275	5930563.96					
AB-SWMH-A02	1736408.808	5930561.459					
AB-SWMH-A03	1736395.218	5930606.072					
AB-SWMH-A04	1736397.499	5930634.699					
AB-SWMH-A05	1736384.478	5930661.737					
AB-SWMH-A06	1736373.367	5930680.462					
AB-SWMH-A07	1736374.892	5930704.543					
AB-SWMH-A08	1736422.667	5930709.802					
AB-SWMH-A09	1736463.023	5930708.994					
AB-SWMH-B01	1736426.407	5930569.222					
AB-SWMH-B02	1736422.946	5930577.287					
AB-SWMH-B03	1736440.298	5930583.666					
AB-SWMH-B04	1736438.274	5930600.867					
AB-SWMH-B05	1736442.865	5930640.86					
AB-SWMH-B06	1736448.471	5930647.217					
AB-SWMH-C01	1736443.746	5930574.852					
AB-SWMH-D01	1736442.3	5930530.05					
AB-SWMH-D02	1736439.882	5930508.988					
AB-SWMH-E01	1736401.29	5930463.289					
AB-SWMH-E02	1736416	5930461.302					

WASTEWATER FLUSHING PIT

COC	ORDINATES			
POINTS	OINTS EASTING			
AB-WW-FLUSHINGPIT01	1736449.426	5930662.319		

WATER SUPPLY FIRE HYDRANT

	COORDINAT	ES							
POINTS	EASTING	NORTHING							
AB-WS-FH01	1736429.983	5930565.959							
AB-WS-FH02	1736444.512	5930671.851							

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CABRA DEVELOPMENTS LIMITE	D

Conditions of Use;
These drawings shall only be used for the purpose for which they were supplied. Do not measure off this drawing. If doubt please contact Engineer.

DRAWING TITLE

AS BUILT 19-LOT RESIDENTIAL SUBDIVISION
AT 236 MATUA ROAD, HUAPAI
BUN60306658 / ENG60324072

AS BUILT COORDINATE TABLES

DRAWING NUMBER
1302-AB-PG107

DATE REVISION

JUNE 2019 -



Note:

All survey data has been collected and provided to Aspire by C&R Surveyors LTD, Registered Surveyors

I certify that these As-built Plans are an accurate record of the works undertaken and that:

 The Coordinates (X, Y) are in terms of NZTM on NZGD (2000), and are within ± 50mm.

• The **Levels** (Z) are in terms of the Auckland 1946 (MSL) LINZ datum (DOSLI datum), and are within ± 10mm.

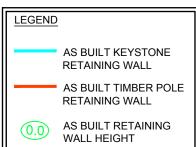


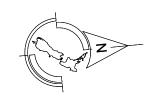
Chartered Professional Engineer

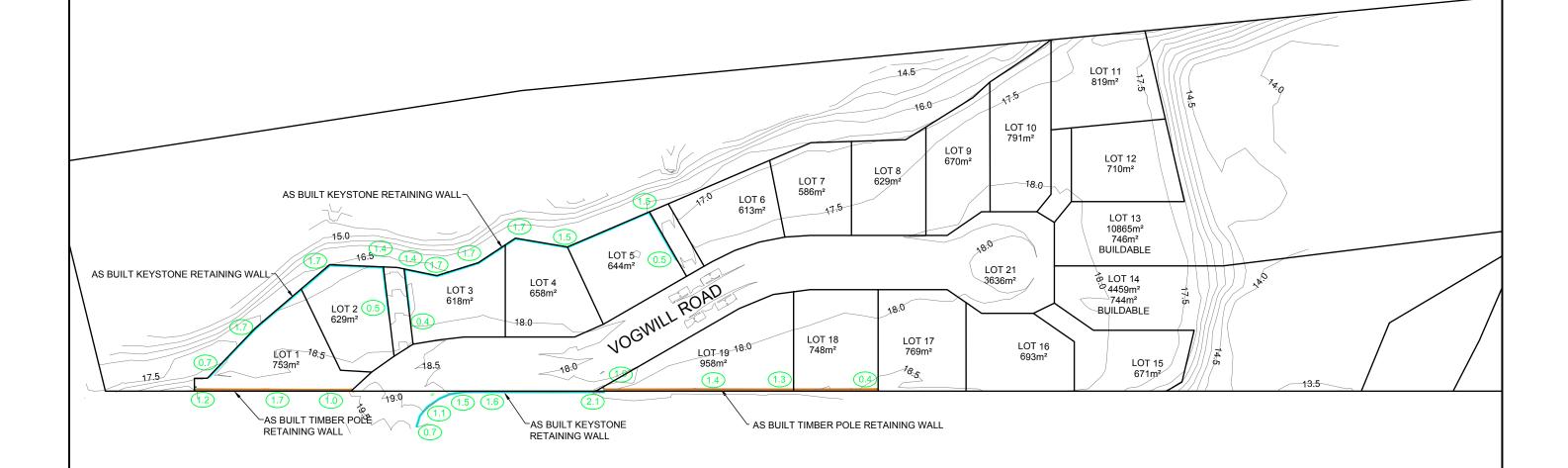
Date: 18 June 2019

Signed:

Name: Evan Alexander Peters Contact Phone: (09) 426 6552 Email: evan@aspeng.co.nz







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CABRA DEVELOPMENTS LIMITED
Conditions of Use; These drawings shall only be used for the purpose for which they were supplied. Do not measure off this drawing. If doubt please contact Engineer.

AS BUILT 19-LOT RESIDENTIAL SUBDIVISION
AT 236 MATUA ROAD, HUAPAI
BUN60306658 / ENG60324072

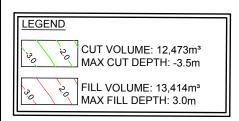
AS BUILT CONTOUR PLAN

1302-AB-EW201

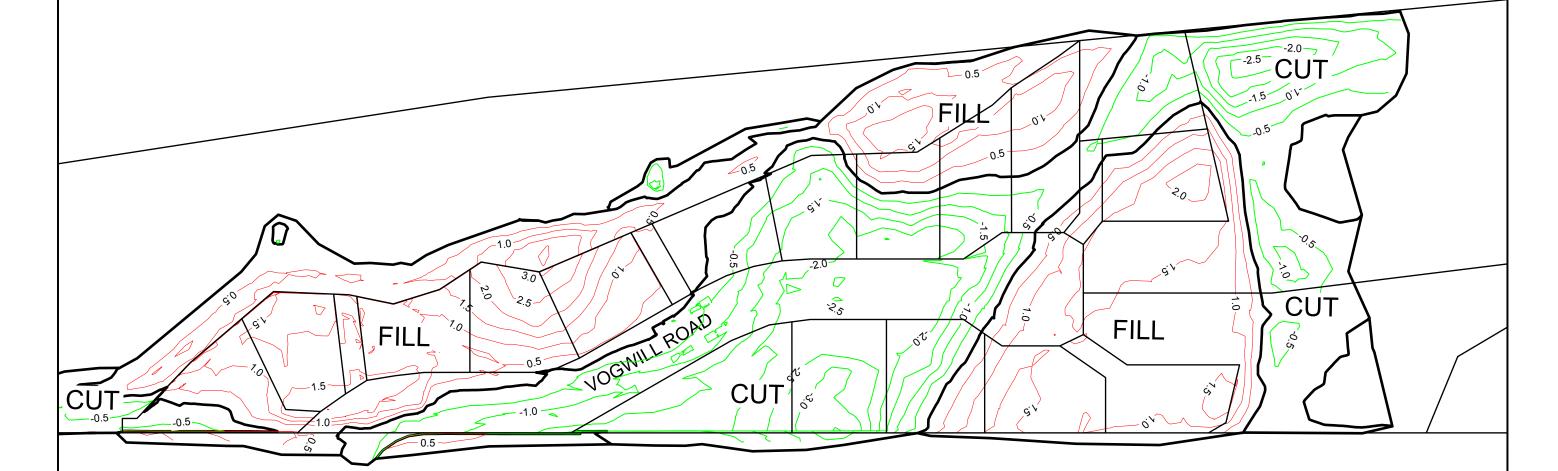
DATE REVISION

JUNE 2019 -









Note

All survey data has been collected and provided to Aspire by C&R Surveyors LTD, Registered Surveyors

I certify that these As-built Plans are an accurate record of the works undertaken and that:

- The Coordinates (X, Y) are in terms of NZTM on NZGD (2000) and are within ± 50mm.
- The **Levels** (Z) are in terms of the Auckland 1946 (MSL) LINZ datum (DOSLI datum), and are within ± 10mm.

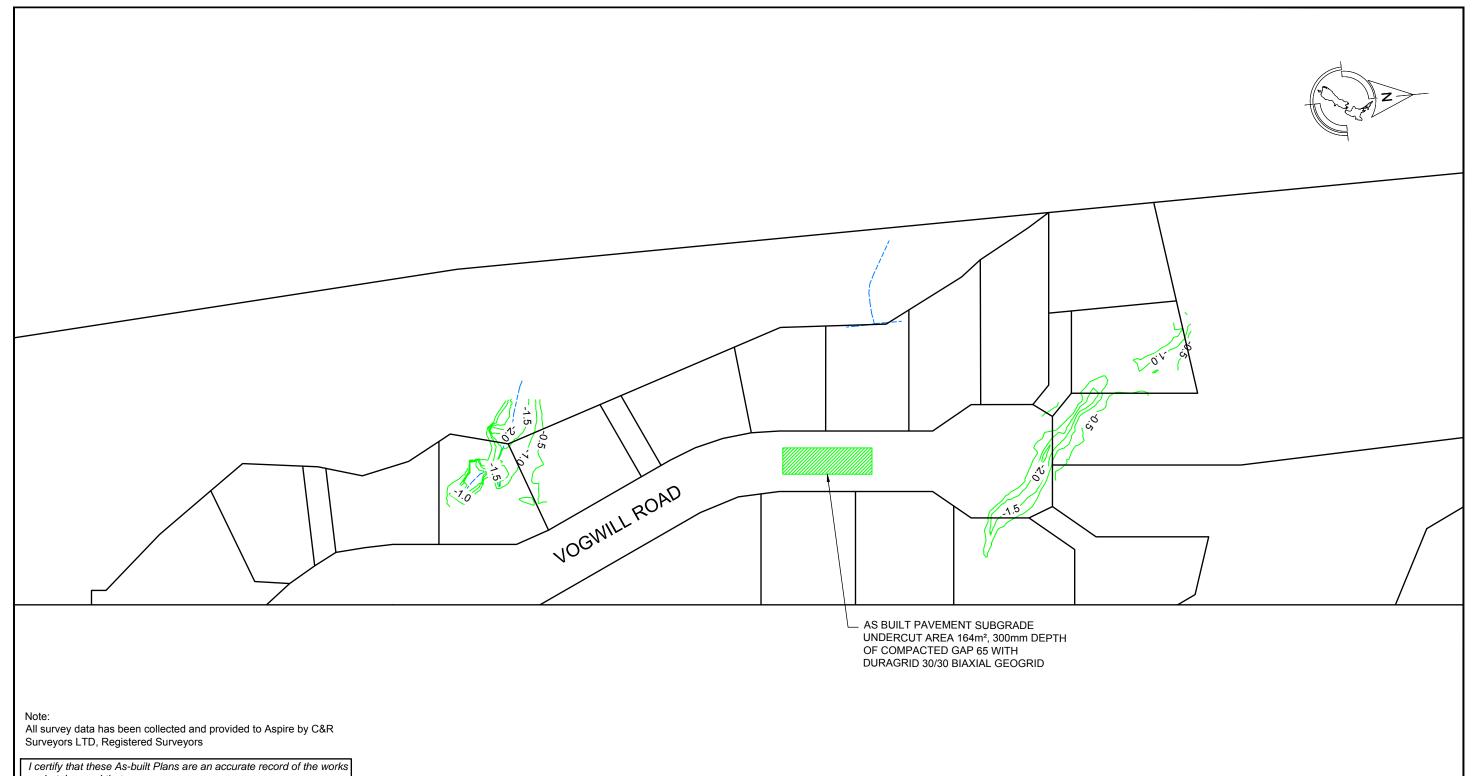
Signed:

Chartered Professional Engineer

Date: 18 June 2019

Name: Evan Alexander Peters Contact Phone: (09) 426 6552 Email: evan@aspeng.co.nz NOTE: ALL LEVELS IN TERMS OF LINZ DATUM: HORIZONTAL DATUM: GEODETIC DATUM 2000 VERTICAL DATUM: AUCKLAND MEAN SEA LEVEL 1946

KEV.	DESCRIPTION CHECK	AFFD DATE SCALE 1.1000 F	II AS		CLIENT		DRAWING NUMBER		
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		DESIGN	KC	06.19	CABRA DEVELOPMENTS LIMITED	AT 236 MATUA ROAD, HUAPAI	1302-AB-EW2	202	
		DRAWN	KC	06.19		BUN60306658 / ENG60324072	DATE	REVISION	CONSULTING ENGINEERS
		DRAWING CHECK	MJ		Conditions of Use:	DRAWING TITLE	DATE	KEVISION	
		DESIGN CHECK	MJ	06.19	These drawings shall only be used for the purpose for which they were	AS BUILT CUT & FILL DEPTH CONTOUR PLAN	JUNE 2019	-	Ph: 09 426 6552
		APPROVED	MJ	06.19	supplied. Do not measure off this drawing. If doubt please contact Engineer.	AO BOILT OUT ATTLE BET THE OUTTOON TEAM			Web: www.aspeng.co.nz



undertaken and that:

- The Coordinates (X, Y) are in terms of NZTM on NZGD (2000), and are within \pm 50mm.
- The **Levels** (Z) are in terms of the Auckland 1946 (MSL) LINZ datum (DOSLI datum), and are within ± 10mm.

Signed:

Chartered Professional Engineer

Date: 18 June 2019

Name: Evan Alexander Peters Contact Phone: (09) 426 6552 Email: evan@aspeng.co.nz

ALL LEVELS IN TERMS OF LINZ DATUM: HORIZONTAL DATUM: GEODETIC DATUM 2000 VERTICAL DATUM: AUCKLAND MEAN SEA LEVEL 1946

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CABRA DEVELOPMENTS LIMITED

These drawings shall only be used for the purpose for which they were supplied. Do not measure off this drawing. If doubt please contact Enginee

AS BUILT 19-LOT RESIDENTIAL SUBDIVISION AT 236 MATUA ROAD, HUAPAI

BUN60306658 / ENG60324072	
DRAWING TITLE	
AS BUILT UNDERCUT DEPTH CONTOUR PLAN	

DRAWING NUMBER				
1302-AB-EW2	203			
DATE	REVISION			

JUNE 2019

LEGEND

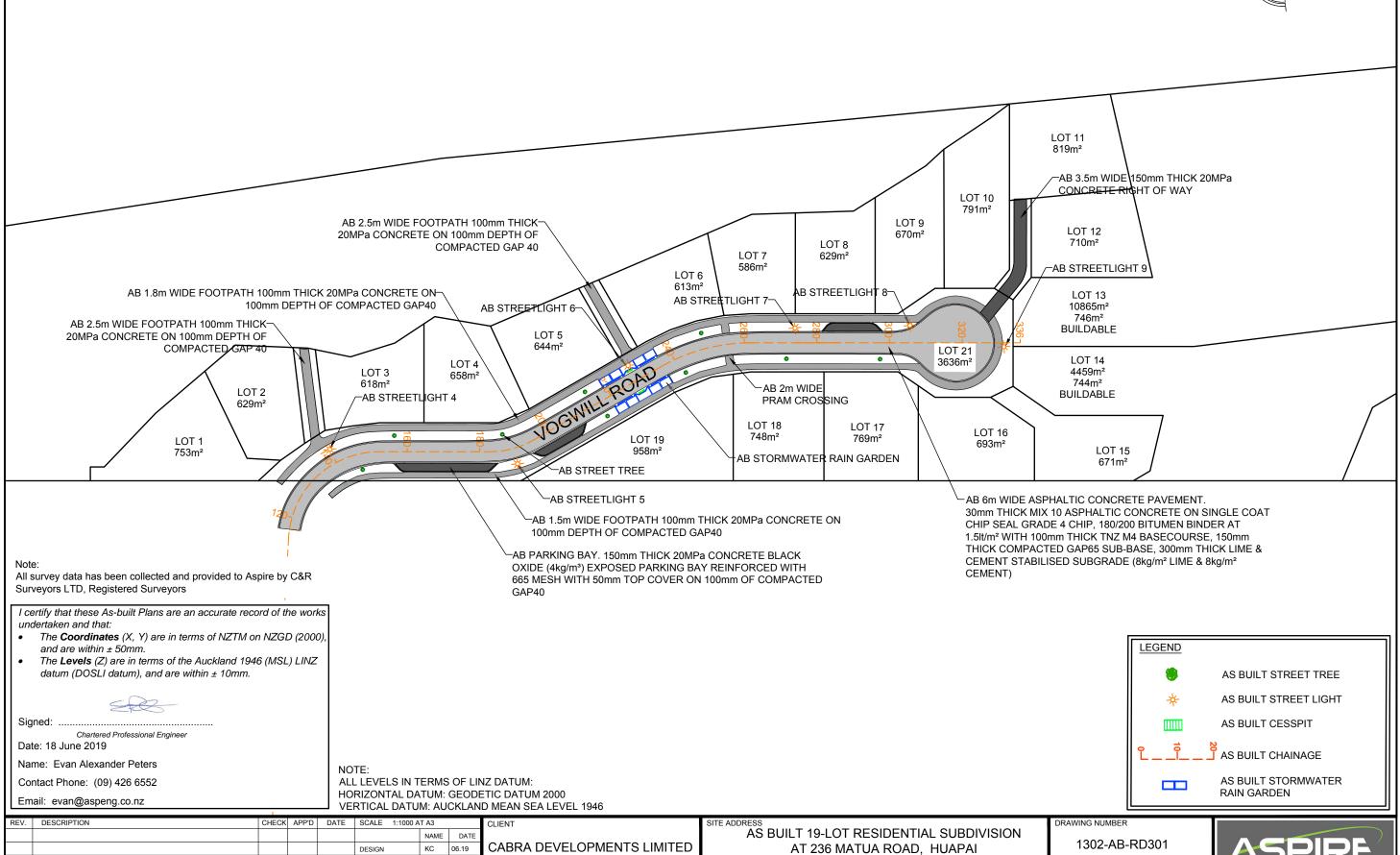
Ph: 09 426 6552 Web: www.aspeng.co.nz

-1.0 UNDERCUT DEPTH CONTOUR

- AS-BUILT UNDERFILL DRAIN

AS-BUILT UNDERCUT AREA





DRAWING TITLE

BUN60306658 / ENG60324072

AS BUILT ROADING PLAN

DATE

JUNE 2019

REVISION

Ph: 09 426 6552

Web: www.aspeng.co.nz

KC 06.19

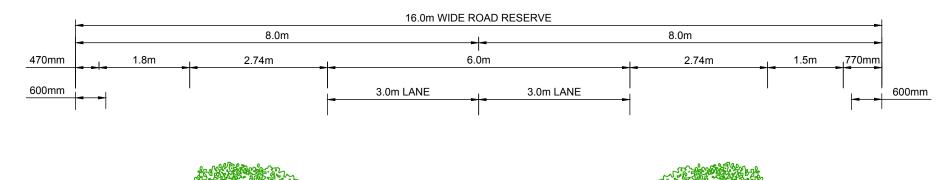
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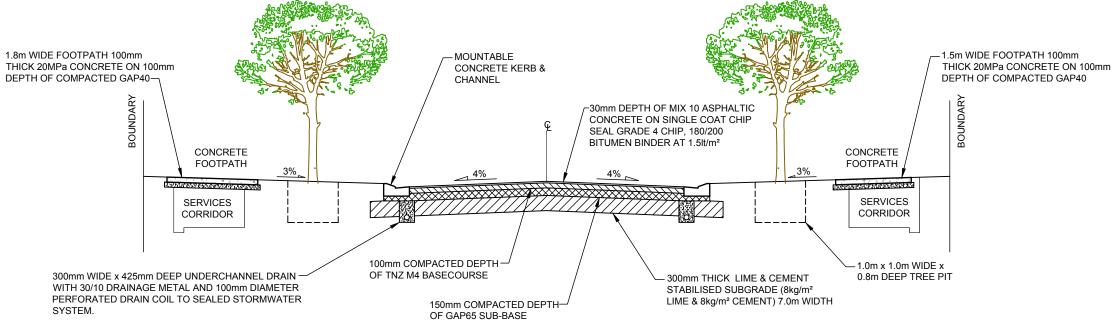
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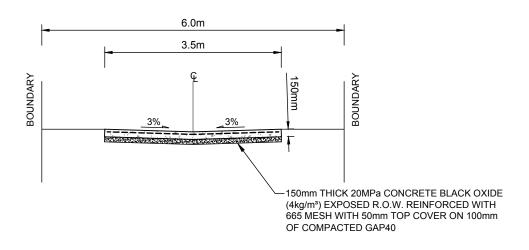
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AS BUILT 6.0m WIDE ROAD CROSS SECTION



AS BUILT 3.5m WIDE R.O.W. CROSS SECTION

REV.	DESCRIPTION	CHECK	APP'D	DATE	SCALE 1:75 AT A3			CL
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9	Conditions of Use:
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^	supplied. Do not measure off this drawing. If doubt please contact Engineer.

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AT 236 MATUA ROAD, HUAPAI
BUN60306658 / ENG60324072

AS BUILT ROAD CROSS SECTION-1

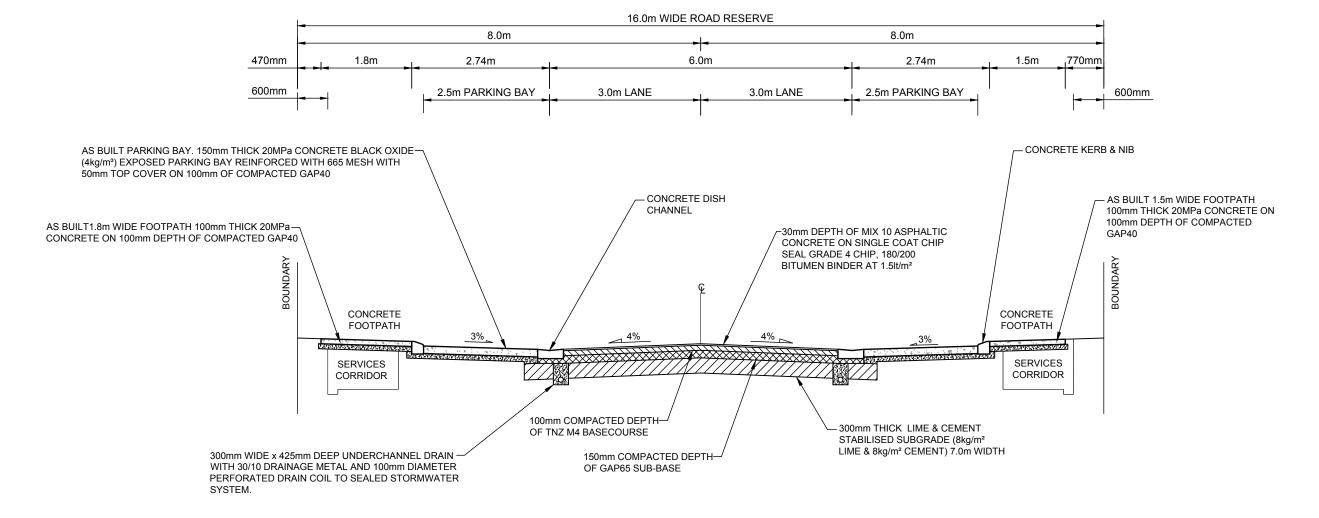
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1302-AB-RD302

DATE REVISION

JUNE 2019 -





AS BUILT 6.0m WIDE ROAD CROSS SECTION (WITH PARKING BAYS)

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-	CABRA DEVELOPMENTS LIMITED
	Conditions of Use; These drawings shall only be used for the purpose for which they were supplied. Do not measure off this drawing. If doubt please contact Engineer.

AS BUILT 19-LOT RESIDENTIAL SUBDIVISION
AT 236 MATUA ROAD, HUAPAI
BUN60306658 / ENG60324072

AS BUILT ROAD CROSS SECTION-2

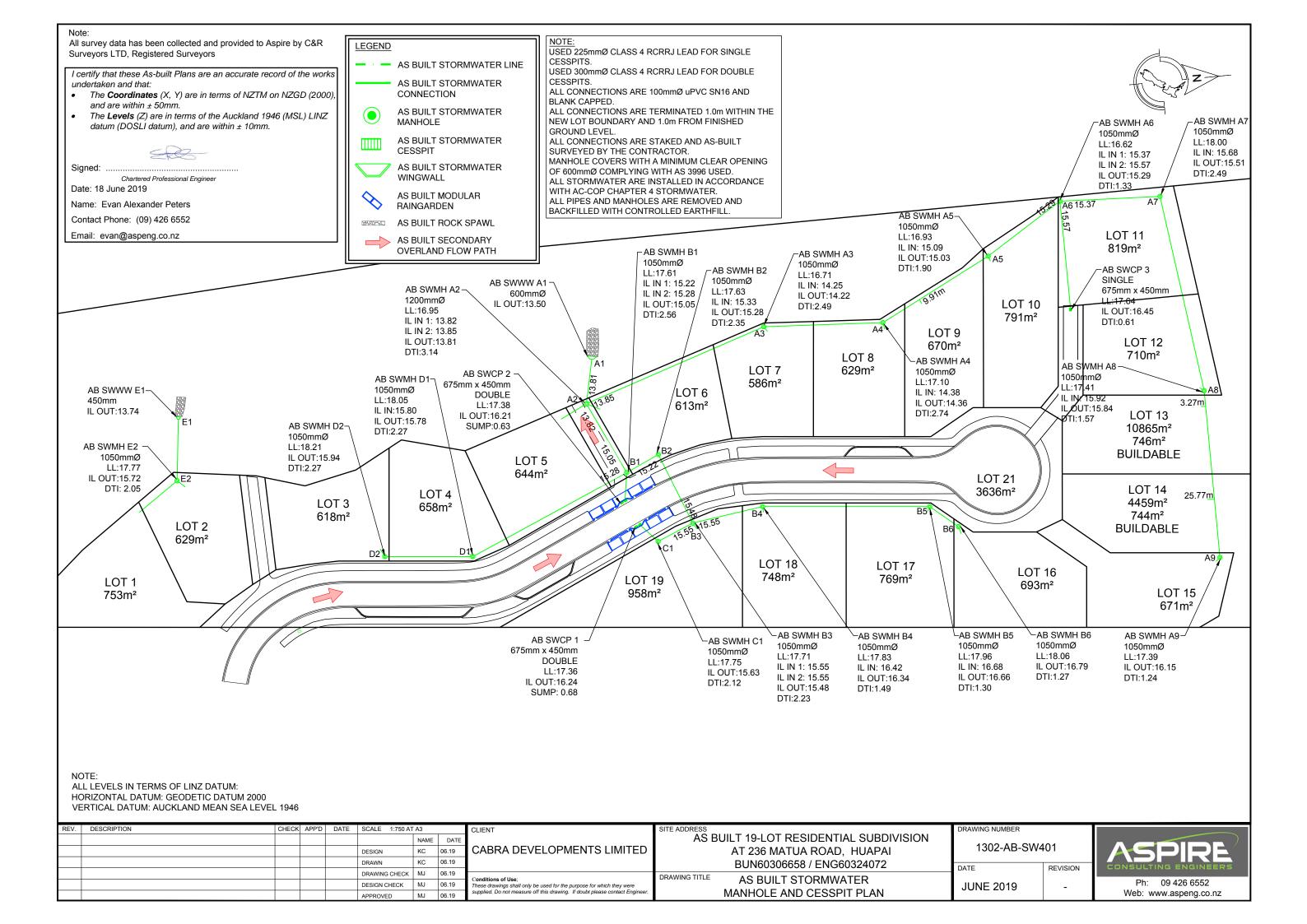
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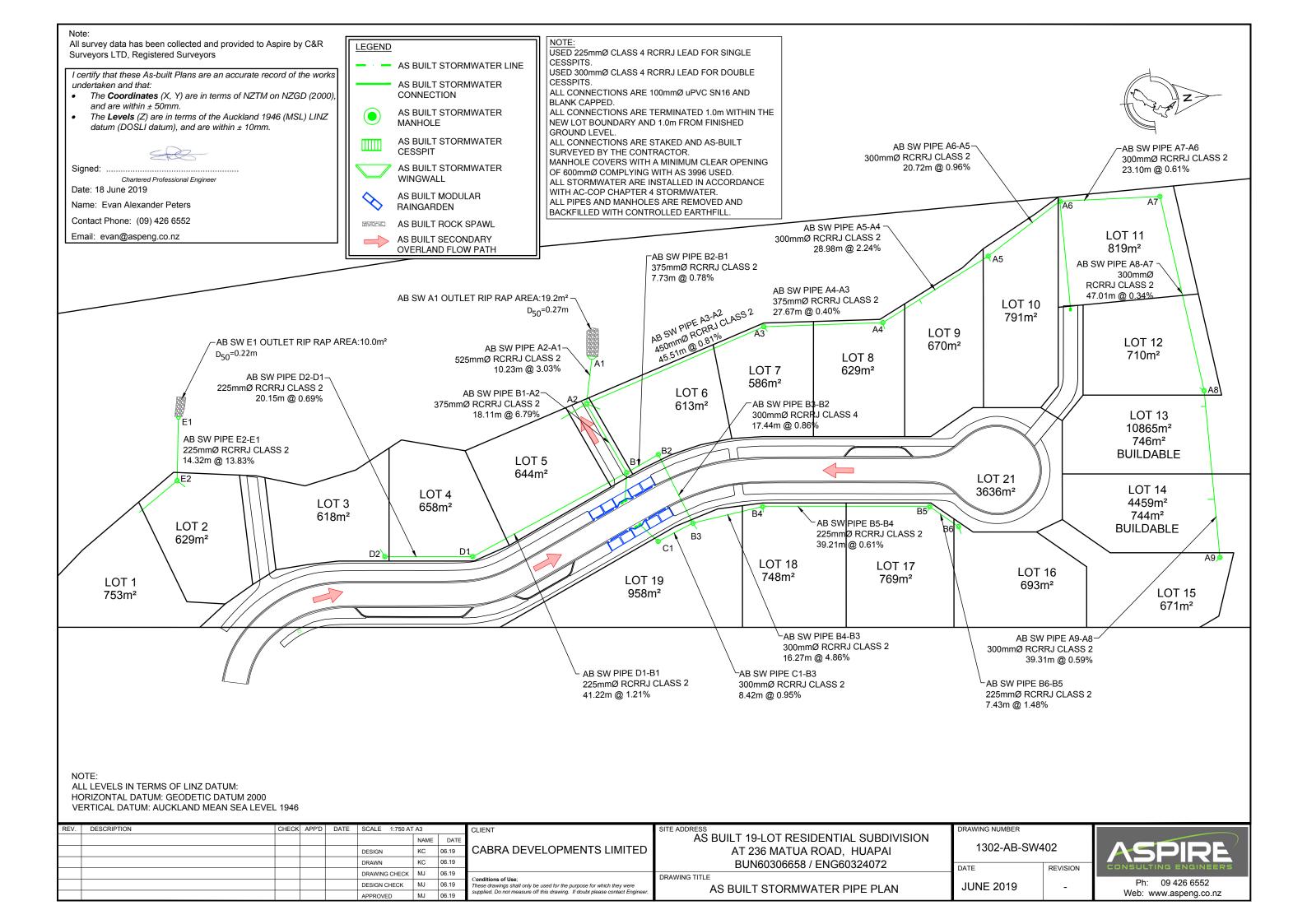
DATE REVISION

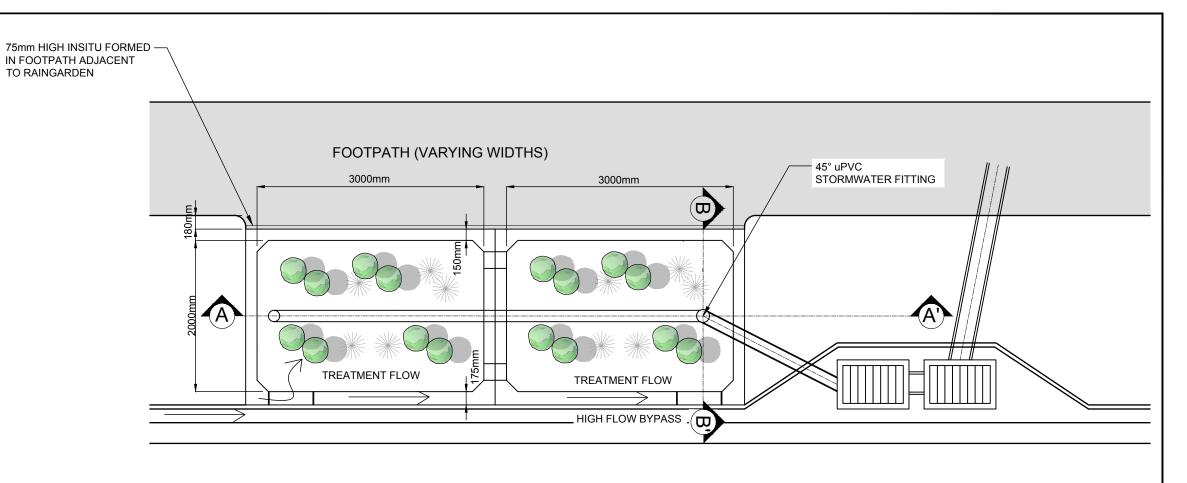
DRAWING NUMBER

JUNE 2019

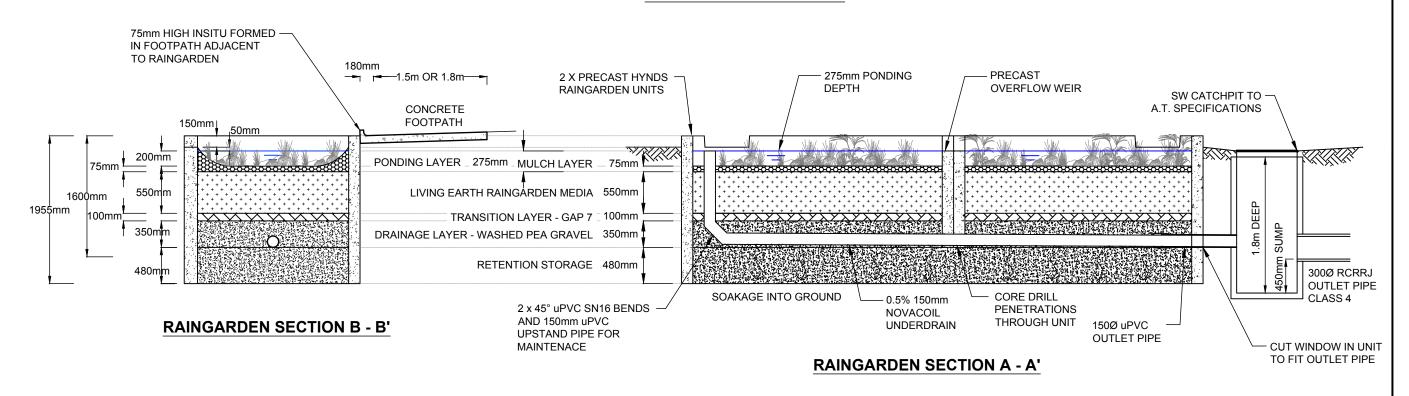






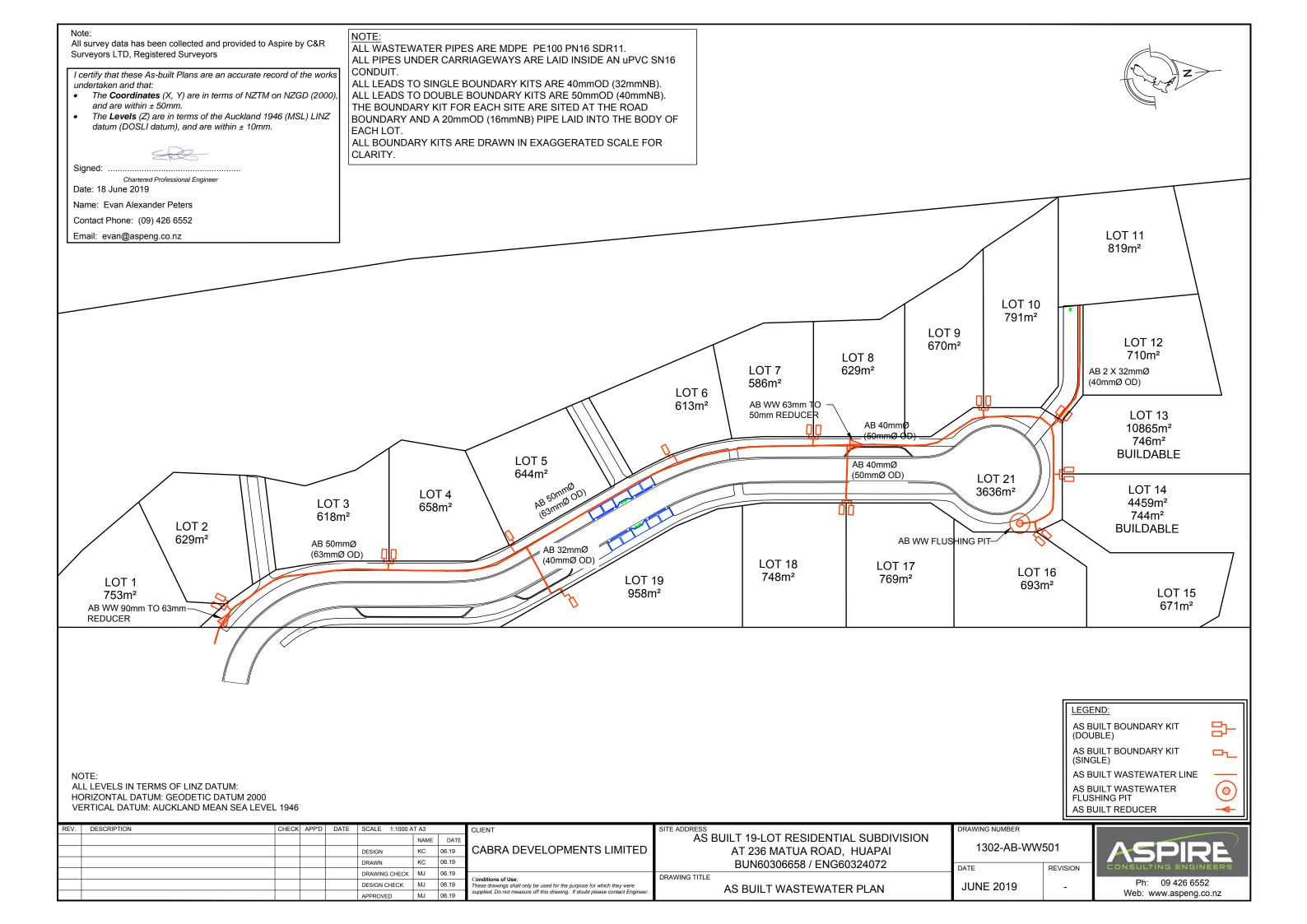


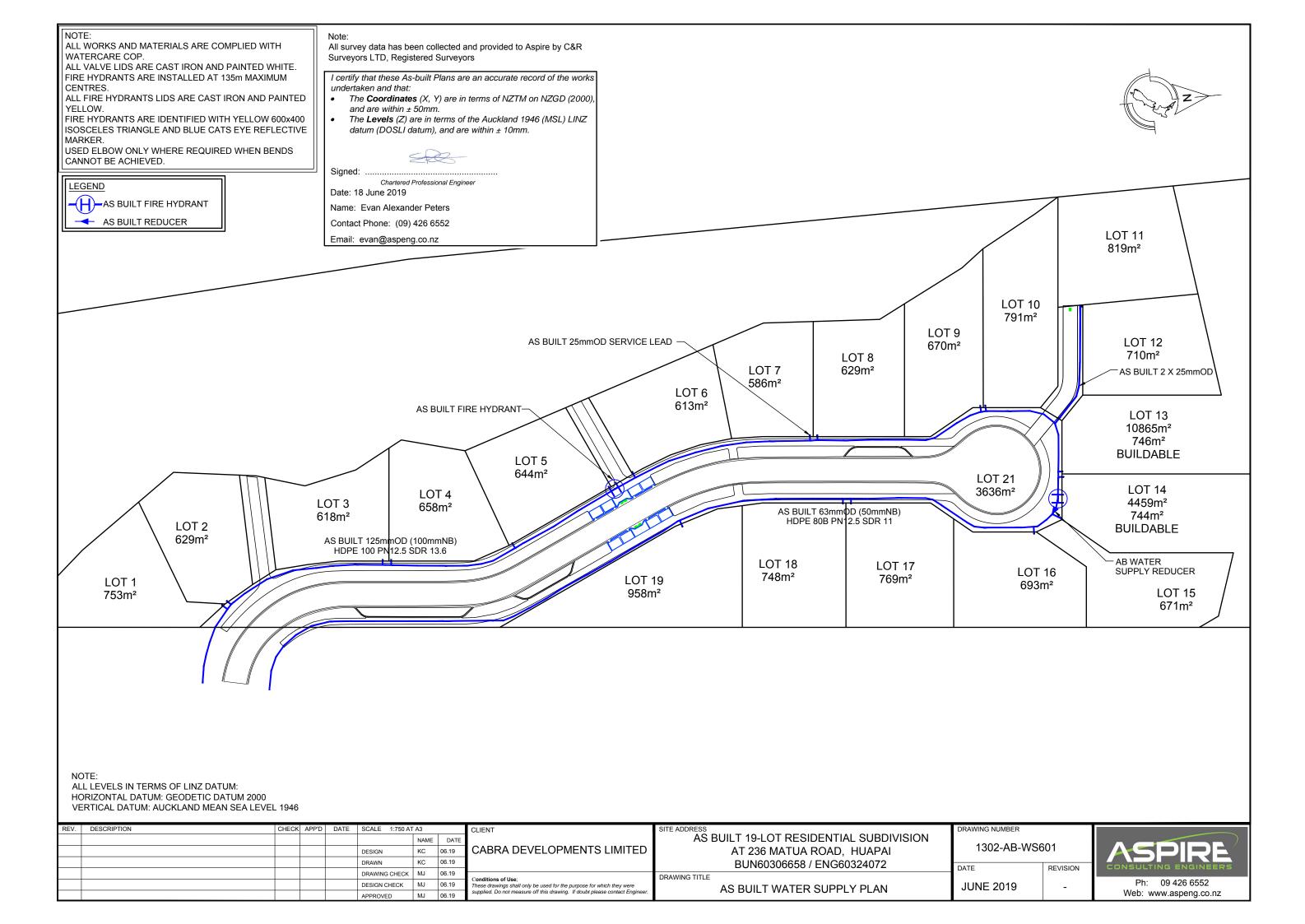
RAINGARDEN PLAN VIEW

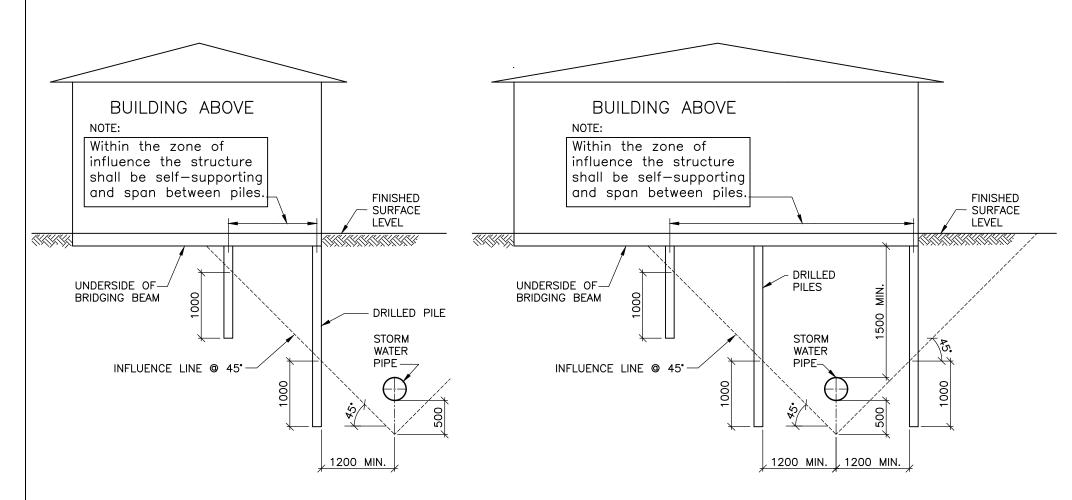


NOTE: ALL LEVELS IN TERMS OF LINZ DATUM: HORIZONTAL DATUM: GEODETIC DATUM 2000 VERTICAL DATUM: AUCKLAND MEAN SEA LEVEL 1946

REV.	DESCRIPTION	CHECK	APP'D DATE	SCALE 1:50 AT A3			CLIENT	SITE ADDRESS	DRAWING NUMBER		
					NAME	DATE		AS BUILT 19-LOT RESIDENTIAL SUBDIVISION	1302-AB-SW403		
				DESIGN	KC	06.19	CABRA DEVELOPMENTS LIMITED	AT 236 MATUA ROAD, HUAPAI			
				DRAWN	KC	06.19		BUN60306658 / ENG60324072	DATE	REVISION	CONSULTING ENGINEERS
				DRAWING CHECK	MJ	06.19	Conditions of Use:	DRAWING TITLE	JUNE 2019	-	COMPONING ENGINEERS
				DESIGN CHECK	MJ	06.19	These drawings shall only be used for the purpose for which they were	AS BUILT STORMWATER RAINGARDEN PLAN			Ph: 09 426 6552
				APPROVED	MJ	06.19	supplied. Do not measure off this drawing. If doubt please contact Engineer.	AO BOILT OTORWWATER TRAINOARDENT EAN			Web: www.aspeng.co.nz







NOTES

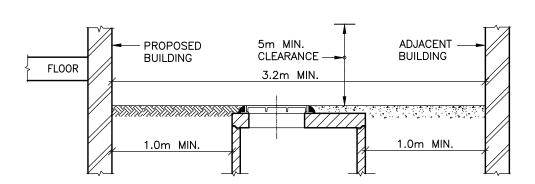
- 1. The information on this page is intended to show examples of typical scenarios and should be used for general guidance purposes only. Significant variations on a site by site basis are to be expected and it is in no way implied that meeting any of the above will guarantee approval.
- 2. Requirements for foundation design etc apply to both sides of pipe.
- 3. No driven piles are permitted within 10m of brick Stormwater Structures, or within 5m of all other Stormwater Structures.
- 4. Specific approval is required from Auckland Council for driven piles in partially drilled holes, within the 5m/10m zone.
- 5. Piles that may be required to resist horizontal forces will require specific design.
- 6. Pile/Footing location point must be below 45° "Zone of Influence".
- 7. All Manholes shall have 24 hours unobstructed access.
- 8. Manholes in basements, or where sufficient clearance is unavailable, are not permitted.
- 9. All pipe buildovers will require approval by Auckland Council.
- 10. Refer to section 4.3.21 for pipe buildover requirements.

"BUILD CLOSE" NOTES:

- 1. Specific approval is required from Auckland Council if building adjacent to pipes, larger than 375mm internal diameter, or greater than 3.0m of depth.
- 2. Building to be outside all overland flow paths and floodplains.
- 3. Pile constructed to a depth of 1.0m below influence line.
- 4. Outside zone of influence, normal foundation requirements apply.

"BUILD OVER" NOTES:

- 1. Applies to stormwater pipes 375mm nominal diameter or less.
- 2. Bridging over pipes larger than 375mm nominal diameter is NOT allowed under any circumstances.
- 3. Pile constructed to a depth of 1.0m below influence line.
- 4. Outside zone of influence, normal foundation requirements apply.
- 5. Bridging is NOT allowed over pipes where clear vertical seperation distance from top of pipe to underside of bridging beam is less than 1.5m



MANHOLE CONSTRUCTION CLEARANCE

STORMWATER ENGINEERING STANDARD DETAILS ISSUE/REVISION: 1 DATE: 30 September 2013 CAD FILE: AC-STD-SW22 AUGKLAND GOUNGIL

ENVIRONMENTAL-SW

Auckland •

Council

ORIGINAL
SCALE: AS NOTED

Sht DRAWING No. REV

ACSD

SW22

STORMWATER PIPE AND MANHOLE CONSTRUCTION CLEARANCE REQUIREMENTS

MANHOLES NEAR BUILDINGS AND BUILDING CLOSE OR OVER PIPES

Appendix C: Laboratory Test Data



DETERMINATION OF THE WATER CONTENT, LIQUID LIMIT & LINEAR SHRINKAGE TEST METHOD NZS 4402 : 1986 TEST 2.1, 2.2 & 2.6

Project Name: 236 Matua Road, Huapai

Project No :
Client : CMW Geosciences Ltd Page :

Client: CMW Geosciences Ltd Page: 1of 1
Address: PO Box 300206 Date of Order: 26.03.19

Albany, Auckland 0754

Sample Method : Hand Auger

Attention: Trent Knox Sample Date: 22.03.19

Sampled By: CMW Geosciences Ltd

19 0092 00

Test Details:

Test performed on : Whole Sample

History: Natural

Sample No.	Location	Depth (m)	Liquid Limit	Linear Shrinkage	Natural Water Content (%)
3071	Lot 7	0.4 to 0.8	74	20	41.1
3081	Lot 3	0.4 to 0.8	60	19	28.9
3091	Lot 14	0.4 to 0.8	68	20	28.9
3101	Lot 18	0.5 to 0.9	57	14	41.4

Comments:

 Tested By:
 EC & SN
 Date :
 29 & 01.04.19

 Calculated By :
 ZH
 Date :
 03.04.19

 Checked By :
 ZH
 Date :
 04.04.19

Appendix D: Field Test Data



236 Matua Road

AKL2016-0630LAA Rev.0

AKL2016-0630

29/05/2019

Huapai

Cabra

Project:

Project No:

Location:

Report No:

Client:

Report Date:

Client Address:

Client Reference:

LF11 Rev.8 Soil Field Density NDM Direct Transmission with VSS Report (Cohesvie Soils)

Auckland Laboratory

CMW Geosciences (NZ) Ltd Partnership Building C, 9 Piermark Drive, Rosedale, NZ 0632

PO Box 300206, Albany, Auckland, NZ 0752

Phone: +64 (09) 4144 632

Test Methods:

Notes:

Assumed

NZS 4402.2.1:1986 NZS 4407.4.2.2:2015

Testing Locations Selected By:

CMW Field Staff

NZGS:August 2001 (1) Blade size of 19mm used.

ACCREDITED LABORATORY

Tests indicated as not accredited are outside the scope of the laboratory's accreditation

Solid Density:

Measurements marked * are not accredited and are outside the scope of the laboratories accreditation

CHETTE RETERE	icc.																			
				Van	e ID	li	n-situ Va	ne Shear	Strength	ıs				Field and	Laboratory Te	esting Data				
Date Sampled	Sample No.	Test Location*	Soil Description*	Head #	Blade #	Test 1 (kPa)	Test 2 (kPa)	Test 3 (kPa)	Test 4 (kPa)	Ave.	Gauge Wet Density (t/m³) **	Gauge Dry Density (t/m³)	Gauge Water Content (%)	Gauge Air Voids (%)	Gauge Probe Depth	Oven Water Content (%)	Solid Density (t/m³) *	Oven Dry Density (t/m³)	Calculated Air Voids (%) *	Comments
9/10/2018	N1	Wallway Area	CLAY	1589	1589	185	185	215+	UTP	195+	1.94	1.53	26.2	3	300	25.3	2.70	1.54	4	
11/10/2018	N2	Lot 14	CLAY	1589	1589	UTP	UTP	215	215	215+	1.99	1.47	30.0	1	300	31.3	2.70	1.52	-4	
18/10/2018	N3	Gully Backfill	CLAY	1589	1589	UTP	UTP	215	UTP	215+	1.82	1.40	30.2	6	300	27.5	2.70	1.42	8	
23/10/2018	N4	Refer to site plan	CLAY	1589	1589	UTP	UTP	UTP	UTP	UTP	1.90	1.51	25.4	3	300	22.9	2.70	1.54	8	
	N5	Refer to site plan	CLAY	1589	1589	UTP	UTP	UTP	UTP	UTP	1.88	1.49	26.4	6	300	22.6	2.70	1.54	9	
	N6	Refer to site plan	CLAY	1589	1589	UTP	UTP	215+	UTP	215+	1.89	1.45	28.4	5	300	29.4	2.70	1.46	3	
	N7	Refer to site plan	CLAY	1589	1589	UTP	UTP	UTP	215+	215+	1.89	1.48	27.8	4	300	29.2	2.70	1.46	3	
1/03/2019	N8	Near East RW boundary	CLAY	1589	1589	UTP	UTP	UTP	UTP	UTP	1.88	1.49	26.2	6	300	26.8	2.70	1.48	5	
	N9	Near East RW boundary	CLAY	1589	1589	UTP	UTP	UTP	UTP	UTP	1.88	1.51	24.7	7	300	21.3	2.70	1.54	10	
3/05/2019	N10	Silt Pond Backfill	CLAY	1620	1620	197	197	197	197	197	1.84	1.41	29.9	5	250	28.5	2.70	1.42	6	
	N11	Silt Pond Backfill	CLAY	1620	1620	197	197	197	197	197	1.83	1.39	32.4	3	300	30.0	2.70	1.42	5	

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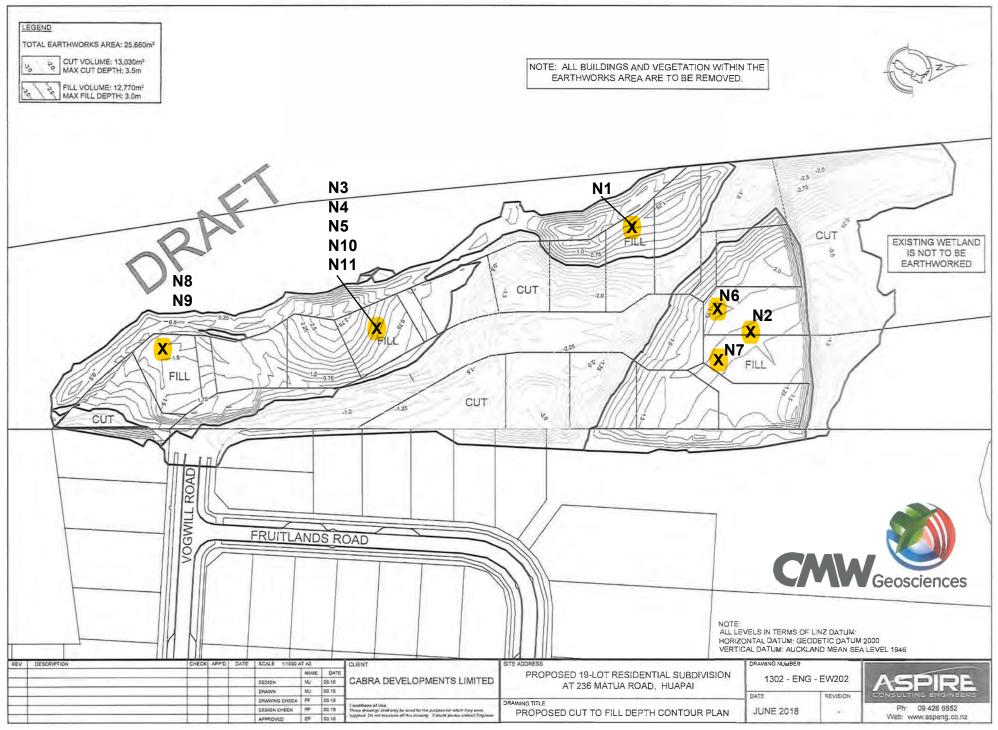
** Gauge Wet Densities outside of the calibrated range of 1.728 to 2.756 t/m³ are not accredited and are outside the laboratories scope of accreditation.

 Created By:
 JLM
 Date:
 15/10/2019

 Checked By:
 JLM
 Date:
 29/05/2019

 Authorised Signatory:
 JMJ
 Date:
 29/05/2019

Page: 1 of 2





Project:

Project No:

Location:

Client:

Report No:

Report Date:

Client Address:

LF10 Rev.13 Soil Core Sample with VSS Density Report

Auckland Laboratory CMW Geosciences (NZ) Ltd Partnership

Building C, 9 Piermark Drive, Rosedale, NZ 0632 PO Box 300206, Albany, Auckland, NZ 0752

Phone: +64 (09) 4144 632

Test Methods: NZS 4402:1986 5.1.3

Notes: Solid Density:

Assumed

AKL2016-0630 Huapai AKL2016-0630LAB Rev.1

236 Matua Road

NZS 4402:1986 2.1 NZGS:August 2001 Testing Locations Selected By: 1 Blade size of 19mm used.

CMW Field Staff

4/06/2019

Cabra

Tests indicated as not accredited are outside the scope of the laboratory's accreditation

Measurements marked * as not accredited are outside the scope of the laboratories accreditation

Client Reference:											ACCRED	ITED LABORA	TORY	atorg s accredit	ation		
				Vane ID		In-situ Vane Shear Strengths					Field and	Laboratory Te					
Date Sampled	Sample No.	Test Location*	Soil Description*	Head #	Blade #	Test 1 (kPa)		Test 3 (kPa)	Test 4 (kPa)	Ave.	Bulk Density (t/m³)		Water Content (%)	Solid Density (t/m³) *	Calculated Air Voids (%) *	Comments	
22/11/2019	C1	Refer to site plan	CLAY	1589	1589	215	UTP	169	159	181+	1.82	1.28	41.6	2.70	-1.3		
	C2	Refer to site plan	CLAY	1589	1589	159	163	166	159	162	1.84	1.30	41.9	2.70	-2.6		
	C3	Refer to site plan	CLAY	1589	1589	157	151	139	160	152	1.94	1.56	25.0	2.70	3.5		

This report should only be reproduced in full.

JLM Created By: Checked By: JLM

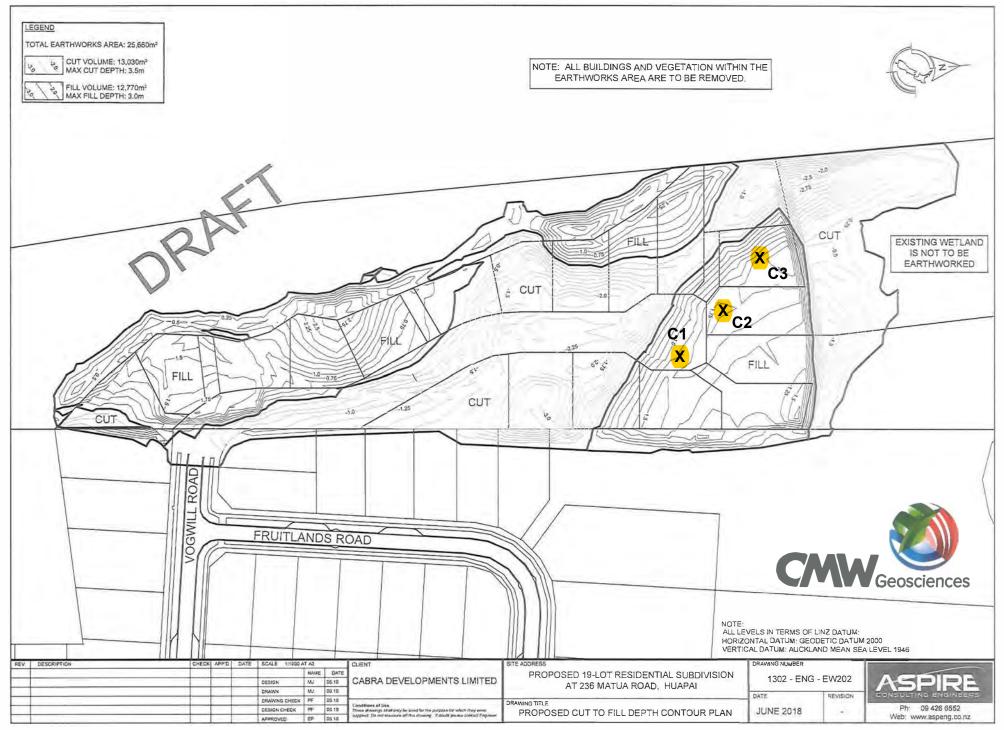
Authorised Signatory:

JLM

26/11/2019 Date: 29/05/2019 Date: Date: 4/06/2019

Page:

1 of 2





236 Matua Road Project:

AKL2016-0630 Project No:

Location: Huapai

Report No: AKI 2016-0630LAC Rev.0

Test Date: 3/12/2018 & 11/12/2018

Tested By: ммс Client: Cabra

Client Address:

Created by:

Checked by:

Authorised Signatory:

JMJ

Auckland Laboratory

CMW Geosciences (NZ) Ltd Partnership Building C, 9 Piermark Drive, Rosedale, NZ 0632 PO Box 300206, Albany, Auckland, NZ 0752

Phone: +64 (09) 4144 632

Testing Locations Selected By:

CMW Field Staff



*Equivalent CBR values calculated using AUSTROADS (2010) Guide to Pavement Technology Part 2, Figure 5.3, For Fine Grained Cohesive Soils, and

are relevant to fine grained cohesive soils only.

Page 1 of 5

accredited and are outside the scope of the laboratory's accreditation

Test No		1		2	:	3		1	5		
Test Location	Vogwi	II Road	Vogwi	II Road	Vogwi	ll Road	Vogwi	ll Road	Vogwi	II Road	
Chainage & Offset	СН	150	CH1	160R	CH1	170L	CH1	.80R	CH1	190L	
Material & Layer	CLA	Y/SG	CLA	Y/SG	CLA	Y/SG	CLAY/SG		CLA	Y/SG	
Depth (mm)	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	
0 - 100	5	10	7	15	6	13	4	8	3	6	
100 - 200	5	10	7	15	5	10	3	6	3	6	
200 - 300	4	8	4	8	2	4	3	6	2	4	
300 - 400	2	4	2	4	3	6	2	4	3	6	
400 - 500	2	4	1	2	3	6	2	4	3	6	
500 - 600	2	4	2	4	2	4	2	4	3	6	
600 - 700	3	6	2	4	3	6	2	4	2	4	
700 - 800	2	4	3	6	3	6	2	4	3	6	
800 - 900	2	4	3	6	3	6	2	4	3	6	
900 - 1000											
Test No		6	,	7	:	8	9	9	10		
Test Location	on Vogwill Road		Vogwi	ll Road	Vogwi	ll Road	Vogwi	ll Road	Vogwill Road		
Chainage & Offset	CH2	200R	CH2	210L	CH2	220R	CH230L		CH2	240R	
Material & Layer						VICC	CLA	Y/SG	CLAY/SG		
material & Layer	CLA	Y/SG	CLA	Y/SG	CLA	1/30	CLA	1/30	CLA		
Depth	CLA Blow Count	Y/SG Equiv CBR*	CLA Blow Count	Y/SG Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	
										Equiv CBR*	
Depth	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	,	
Depth 0 - 100	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	20+	
Depth 0 - 100 100 - 200	Blow Count 4 3	Equiv CBR* 8	Blow Count 3	Equiv CBR* 6 4	Blow Count 4 6	Equiv CBR* 8 13	Blow Count 6 4	Equiv CBR* 13	Blow Count 10 7	20+	
Depth 0 - 100 100 - 200 200 - 300	Blow Count 4 3 2	Equiv CBR* 8 6 4	Blow Count 3 2 3	Equiv CBR* 6 4 6	Blow Count 4 6 4	8 13 8	Blow Count 6 4 3	Equiv CBR* 13 8 6	Blow Count 10 7 3	20+ 15 6	
Depth 0 - 100 100 - 200 200 - 300 300 - 400	Blow Count 4 3 2	Equiv CBR* 8 6 4	Blow Count 3 2 3 3	6 4 6 6	Blow Count 4 6 4 3	8 13 8 6	Blow Count 6 4 3 3	13 8 6	10 7 3 1	20+ 15 6	
Depth 0 - 100 100 - 200 200 - 300 300 - 400 400 - 500	Blow Count 4 3 2 2 2	8 6 4 4 4	Blow Count 3 2 3 3 3	Equiv CBR* 6 4 6 6 6	8low Count 4 6 4 3 3	8 13 8 6 6	6 4 3 3 3 3	Equiv CBR* 13 8 6 6 6	10 7 3 1 2	20+ 15 6 2	
Depth 0 - 100 100 - 200 200 - 300 300 - 400 400 - 500 500 - 600	Blow Count 4 3 2 2 2 2	8 6 4 4 4 4 4	3 2 3 3 3 2 2	6 4 6 6 4 4	8low Count 4 6 4 3 3 2	8 13 8 6 6 4	8low Count 6 4 3 3 4	Equiv CBR* 13 8 6 6 8	10 7 3 1 2 2 2	20+ 15 6 2 4	
Depth 0 - 100 100 - 200 200 - 300 300 - 400 400 - 500 500 - 600 600 - 700	Blow Count 4 3 2 2 2 3	8 6 4 4 4 4 6 6	3 2 3 3 2 4	Equiv CBR* 6 4 6 6 6 4 8	8low Count 4 6 4 3 3 2 2	8 13 8 6 6 4 4	8low Count 6 4 3 3 4 4 3	Equiv CBR* 13 8 6 6 8 6	10 7 3 1 2 2 2 3 3	20+ 15 6 2 4 4	

Date:

Date:

Date:

29/05/2019

29/05/2019

29/05/2019



236 Matua Road Project:

AKL2016-0630 Project No:

Location: Huapai

Report No: AKI 2016-0630LAC Rev.0

Test Date: 3/12/2018 & 11/12/2018

Tested By: ммс Client: Cabra

Client Address:

Auckland Laboratory

CMW Geosciences (NZ) Ltd Partnership Building C, 9 Piermark Drive, Rosedale, NZ 0632 PO Box 300206, Albany, Auckland, NZ 0752

Phone: +64 (09) 4144 632

Testing Locations Selected By:

CMW Field Staff



accredited and are outside the scope of the laboratory's accreditation

Test No	1	.1	1	12	1	.3	1	14	15		
Test Location	Vogwi	ll Road	Vogwi	ill Road	Vogwi	II Road	Vogwi	ill Road	Vogwi	II Road	
Chainage & Offset	Offset CH250L			260R	CH2	270L	CH2	280R	CH2	290L	
Material & Layer	CLA	Y/SG	CLA	Y/SG	CLA	Y/SG	CLAY/SG		CLA	Y/SG	
Depth (mm)	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	
0 - 100	4	8	3	6	3	6	6	13	3	6	
100 - 200	4	8	6	13	3	6	3	6	3	6	
200 - 300	2	4	4	8	1	2	4	8	3	6	
300 - 400	4	8	2	4	1	2	1	2	0	0	
400 - 500	3	6	1	2	1	2	1	2	1	2	
500 - 600	2	4	1	2	1	2	1	2	3	6	
600 - 700	3	6	3	6	1	2	1	2	1	2	
700 - 800	3	6	3	6	1	2	2	4	2	4	
800 - 900	3	6	2	4	2	4	2	4	2	4	
900 - 1000											
Test No	1	.6	1	17	1	18	1	19	20		
Test Location	Vogwi	ll Road	Vogwi	ill Road	Vogwi	ll Road	Vogwi	ill Road	Vogwill Road		
Chainage & Offset	CH3	300R	CH3	310L	CH3	320R	CH130L		CH140R		
Material & Layer	CLA	Y/SG	CLA	Y/SG	CLA	CLAY/SG LS CLAY/SG		LS CLAY/SG		AY/SG	
Depth	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	
0 - 100	5	10	5	10	4	8	6	13	9	20	
100 - 200	3	6	6	13	3	6	5	10	7	15	
200 - 300	5	10	3	6	1	2	3	6	3	6	
300 - 400	10+	20+	2	4	1	2	2	4	2	4	
400 - 500			2	4	3	6	2	4	2	4	
500 - 600			3	6	4	8	2	4	3	6	
600 - 700			3	6	3	6	3	6	3	6	
700 - 800			3	6	4	8	3	6	2	4	
800 - 900			4	8	4	8	2	4	2	4	

Created by: Checked by:

Authorised Signatory:

JMJ

29/05/2019 29/05/2019

29/05/2019

Date:

Date:

Date:

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*Equivalent CBR values calculated using AUSTROADS (2010) Guide to Pavement Technology Part 2, Figure 5.3, For Fine Grained Cohesive Soils, and are relevant to fine grained cohesive soils only.

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236 Matua Road Project:

AKL2016-0630 Project No:

Location: Huapai

Report No: AKI 2016-0630LAC Rev.0

Test Date: 3/12/2018 & 11/12/2018

Tested By: ммс Client: Cabra

Client Address:

Created by:

Checked by:

Authorised Signatory:

JMJ

Auckland Laboratory

CMW Geosciences (NZ) Ltd Partnership Building C, 9 Piermark Drive, Rosedale, NZ 0632 PO Box 300206, Albany, Auckland, NZ 0752

Phone: +64 (09) 4144 632

Testing Locations Selected By:

CMW Field Staff



*Equivalent CBR values calculated using AUSTROADS (2010) Guide to Pavement Technology Part 2, Figure 5.3, For Fine Grained Cohesive Soils, and

are relevant to fine grained cohesive soils only.

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* Equivalent CBR Values are not accredited and are outside the scope of the laboratory's accreditation

Test No	2	1	2	.2	2	3	2	4	2	:5		
Test Location	Vogwi	ll Road	Vogwi	ll Road	Vogwi	ll Road	Vogwi	ll Road	Vogwi	ll Road		
Chainage & Offset	CH:	150L	CH1	L60R	CH1	170L	CH1	.80R	CH:	190L		
Material & Layer	LS CL	AY/SG	LS CL	AY/SG	LS CL	AY/SG	LS CLAY/SG		LS CL	AY/SG		
Depth (mm)	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*		
0 - 100	10	20+	7	15	7	15	10	20+	7	15		
100 - 200	5	10	3	6	3	6	3	6	4	8		
200 - 300	2	4	2	4	3	6	2	4	2	4		
300 - 400	3	6	3	6	2	4	2	4	2	4		
400 - 500	3	6	2	4	2	4	1	2	2	4		
500 - 600	3	6	2	4	2	4	2	4	2	4		
600 - 700	3	6	2	4	2	4	2	4	2	4		
700 - 800	3	6	2	4	2	4	2	4	1	2		
800 - 900	3	6	2	4	2	4	2	4	2	4		
900 - 1000												
Test No	2	16	2	.7	2	8	2	9	30			
Test Location	Vogwi	Vogwill Road Vogwill Road		ll Road	Vogwi	ll Road	Vogwi	ll Road	Vogwill Road			
Chainage & Offset	CH2	200R	CH2	210L	CH2	20R	CH230L		CH2	240R		
				/50		AV/CC	LS CLAY/SG		LS CLAY/SG		LS CLAY/SG	
Material & Layer	LS CL	AY/SG	LS CL	AY/SG	LS CL	AY/SG	L3 CL	,				
Material & Layer Depth	LS CL	AY/SG Equiv CBR*	LS CL Blow Count	Equiv CBR*	LS CL	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*		
										Equiv CBR*		
Depth	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count			
Depth 0 - 100	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	18		
Depth 0 - 100 100 - 200	Blow Count 7 4	Equiv CBR* 15	Blow Count 8	Equiv CBR* 18	Blow Count 10 5	Equiv CBR* 20+ 10	Blow Count 8 11	Equiv CBR* 18 20+	Blow Count 8 5	18		
Depth 0 - 100 100 - 200 200 - 300	Blow Count 7 4 2	15 8 4	Blow Count 8 4 3	18 8 6	Blow Count 10 5 3	20+ 10 6	Blow Count 8 11 5	18 20+ 10	Blow Count 8 5 5	18 10 10		
Depth 0 - 100 100 - 200 200 - 300 300 - 400	7 4 2 3	Equiv CBR* 15 8 4 6	Blow Count 8 4 3 2	18 8 6 4	10 5 3 3	20+ 10 6	8 11 5 3	18 20+ 10 6	Blow Count 8 5 4	18 10 10 8		
Depth 0 - 100 100 - 200 200 - 300 300 - 400 400 - 500	8 Blow Count 7 4 2 3 2 2	Equiv CBR* 15 8 4 6 4	8 4 3 2 3 3	Equiv CBR* 18 8 6 4 6	Blow Count 10 5 3 1	Equiv CBR* 20+ 10 6 6 2	8 11 5 3 4	Equiv CBR* 18 20+ 10 6 8	8 5 5 4 2	18 10 10 8 4		
Depth 0 - 100 100 - 200 200 - 300 300 - 400 400 - 500 500 - 600	Blow Count 7 4 2 3 2 2	Equiv CBR* 15 8 4 6 4	8 4 3 2 3 2 2	18 8 6 4 6 4	Blow Count 10 5 3 1 2	Equiv CBR* 20+ 10 6 2 4	8 11 5 3 4 2	Equiv CBR* 18 20+ 10 6 8 4	8 5 5 4 2 2	18 10 10 8 4		
Depth 0 - 100 100 - 200 200 - 300 300 - 400 400 - 500 500 - 600 600 - 700	Blow Count 7 4 2 3 2 2 2	Equiv CBR* 15 8 4 6 4 4	8 4 3 2 3 2 2 2	Equiv CBR* 18 8 6 4 6 4	Blow Count 10 5 3 1 2 3	Equiv CBR* 20+ 10 6 2 4	8 11 5 3 4 2 2 2	Equiv CBR* 18 20+ 10 6 8 4	8 5 5 4 2 2 2 2	18 10 10 8 4 4		

Date:

Date:

Date:

29/05/2019

29/05/2019

29/05/2019



236 Matua Road Project:

AKL2016-0630 Project No:

Location: Huapai

Report No: AKI 2016-0630LAC Rev.0

Test Date: 3/12/2018 & 11/12/2018

Tested By: ммс Client: Cabra

Client Address:

Auckland Laboratory

CMW Geosciences (NZ) Ltd Partnership Building C, 9 Piermark Drive, Rosedale, NZ 0632 PO Box 300206, Albany, Auckland, NZ 0752

Phone: +64 (09) 4144 632

Testing Locations Selected By:

CMW Field Staff



accredited and are outside the scope of the laboratory's accreditation

Test No	3	1	3	32	3	33	3	34	35	
Test Location	Vogwi	ll Road	Vogwi	ill Road	Vogwi	II Road	Vogwi	II Road	Vogwi	II Road
Chainage & Offset	Offset CH250R			260L	CH2	270R	CH2	280R	CH2	290L
Material & Layer	LS CL	AY/SG	LS CL	AY/SG	LS CL	AY/SG	LS CLAY/SG		LS CL	AY/SG
Depth (mm)	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*
0 - 100	6	13	11	20+	4	8	3	6	7	15
100 - 200	3	6	5	5 10		2	0	0	6	13
200 - 300	1	2	18	20+	1	2	1	2	3	6
300 - 400	1	2			1	2	2	4	2	4
400 - 500	1	2			2	4	1	2	1	2
500 - 600	2	4			1	2	1	2	3	6
600 - 700	2	4			1	2	3	6	2	4
700 - 800	2	4			2	4	3	6	2	4
800 - 900	2	4			2	4	2	4	2	4
900 - 1000										
Test No	36		(3)	37	3	88	3	39		
Test Location	Vogwi	Vogwill Road		ill Road	Vogwi	II Road	Vogwi	II Road		
Chainage & Offset	CH3	300R	CH	310R	CH	320L	CH	320R		
Material & Layer	LS CL	AY/SG	LS CL	AY/SG	LS CL	AY/SG	LS CL	AY/SG		
Depth	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*	Blow Count	Equiv CBR*
0 - 100	9	20	8	18	7	15	11	20+		
100 - 200	5	10	3	6	7	15	5	10		
200 - 300	15	20+	2	4	3	6	3	6		
300 - 400			3	6	3	6	3	6		
400 - 500			3	6	1	2	2	4		
500 - 600			3	6	1	2	3	6		
600 - 700			4	8	3	6	3	6		
700 - 800			5	10	3	6	2	4		
800 - 900			5	10	3	6	2	4		

Created by: Checked by:

Authorised Signatory:

JMJ

Date: Date:

Date:

29/05/2019

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*Equivalent CBR values calculated using AUSTROADS (2010) Guide to Pavement Technology Part 2, Figure 5.3, For Fine Grained Cohesive Soils, and are relevant to fine grained cohesive soils only.

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Appendix E: Producer Statements



24 June 2019 Document Ref: AKL2016_0630AI Rev 0

Mr Will Stone Cabra Developments Limited, PO Box 197, Orewa, 0946

Dear Will

RE: CONSTRUCTION REVIEW FOR THE CONSTRUCTION OF TIMBER POLE AND KEYSTONE RETAINING WALLS – BCO10274504-A

236 MATUA ROAD, HUAPAI

1 INTRODUCTION

CMW Geosciences (CMW) has visited the site at 236 Matua Road, legally described as Lot 3 DP 171599 on several occasions between November 2018 and May 2019 to observe the construction of timber pole and segmental block retaining walls.

Our work has included review of the following documents and drawings:

- Conditions of Auckland Council Building Consent referenced BCO10274504-A, issued 17 October 2018;
- Consented construction drawings, prepared by CMW Geosciences, referenced AKL2016-0630 No Fines Concrete Walls dated 8 October 2018 and AKL2016-0630 Timber Pole Retaining Wall Design dated 30 July 2018;
- Eastern keystone retaining walls geotechnical design report for 236 Matua Road prepared by CMW Geosciences, referenced AKL2016-0630AC Rev. 1, dated 8 October 2018;
- Western keystone retaining walls geotechnical design report for 236 Matua Road prepared by CMW Geosciences, referenced AKL2016-0630AD Rev. 1, dated 8 October 2018; and,
- Timber pole retaining wall geotechnical design report for 236 Matua Road prepared by CMW Geosciences, referenced AKL2016-0630AB Rev. 0 dated 30 July 2018.

2 TIMBER POLE RETAINING WALLS

CMW observed the construction of timber post retaining walls on Lots 1, 18 and 19.

The site works observed and/or tested by CMW staff incorporated representative testing and observation of:

- assessment of soil strengths in the exposed foundation excavations;
- pile size and depth;

- pile hole spacing and diameter;
- timber pole size and placement;
- lagging installation; and,
- drainage material and installation

Ground conditions within the pile holes generally consisted of alluvial sediments overlying Waitemata Group deposits. The design parameters prepared by CMW for timber pole retaining walls were used for the construction of the walls and measured vane shear strengths were all in excess of 70kPa. Where groundwater was observed within the pile holes, contractors were requested to pump the water out prior to pouring concrete. Pile dimensions, timber dimensions and drainage works were all in accordance with the design requirements.

3 KEYSONE RETAINING WALLS

CMW observed the construction of keystone retaining walls on Lots 1 to 5 and along the road reserve.

The site works observed and/or tested by CMW staff incorporated representative testing and observation of:

- assessment of soil strengths in exposed foundation excavations;
- block placement;
- no fines concrete width;
- drainage material and installation.

The width of the no fines concrete was observed prior to pouring. On one occasion the no fines concrete width did not meet the required amount. This was reworked and subsequently observed to be correct by CMW.

4 CONCLUSION

On the basis of our observations and testing, we consider that the construction of the timber pole and keystone retaining walls has been undertaken in accordance with the approved Building Consent and related approved documentation described above, is in accordance with the requirements and/or recommendations of the geotechnical report and provides the basis for our attached PS4 Construction Review Producer Statement.

CMW's site presence during construction for this project included periodic observations of specific elements of work as described herein. As we were not on site at all times during construction, we have relied on the Contractor's diligence and construction observations certification to ensure that the works have been carried out in accordance with:

- a) The approved Contract drawings and design details;
- b) The approved Contract specifications;
- c) The conditions of Resource and Building Consents where applicable;
- d) The relevant Geotechnical Investigation reports, recommendations and site instructions;

For and on behalf of CMW Geosciences

Richard Knowles

Principal Geotechnical Engineer, CPEng

Distribution: 1 electronic copy to Aspire Consulting Engineers Limited via email

Original held at CMW Geosciences

Attachments: Producer Statement - Construction Review















Building Code Clause(s)...B.1

PRODUCER STATEMENT - PS4 - CONSTRUCTION REVIEW

(Guidance on use of Producer Statements (formerly page 2) is available at www.engineeringnz.org)

ISSUED BY: CMW Geosciences (NZ) Limited Partnership (Construction Review Firm)
TO: Cabra Developments Limited (Owner/Developer)
TO BE SUPPLIED TO: Auckland Council (Building Consent Authority)
IN RESPECT OF: Construction of Timber Pole and Keystone Retaining Walls (Description of Building Work)
AT: 236 Matua Road, Huapai (Address)
Town/City: LOT.3 DP.171599 SO
We CMW Geosciences (NZ) Limited Partnership have been engaged by Cabra Developments Limited (Construction Review Firm)
To provide CM1 CM2 CM3 CM4 CM5 (Engineering Categories) or observation as per agreement with
owner/developer. Cabra Developments Limited
or other as described in CMW letter referenced AKL2016_0630Al Rev 0 dated 24 June 2019 (Extent of Engagement)
in respect of clause(s) .B1 of the Building Code for the building work described in
documents relating to Building Consent No. BCO10274504 and those relating to
Building Consent Amendment(s) Nos. BCO10274504-A issued during the course of the works. We have sighted these Building Consents and the conditions of attached to them.
Authorised instructions/variations(s) No
On the basis of this review these review(s) and information supplied by the contractor during the course of the works and on behalf of the firm undertaking this Construction Review, I believe on reasonable grounds that or Part only of the building works have been completed in accordance with the relevant requirements of the
Building Consent and Building Consent Amendments identified above, with respect to Clause(s). B1
Richard Knowles (AC Author #2342) am: CPEng 160049 # Reg Arch #
I am a member of: Engineering New Zealand NZIA and hold the following qualifications BE (Civil), CMEngNZ, CPEng The Construction Review Firm issuing this statement holds a current policy of Professional Indemnity Insurance no less than \$200,000*.
The Construction Review Firm is a member of ACENZ:
SIGNED BY Richard Knowles (AC Author #2342) (Name of Construction Review Professional) (Signature)
ON BEHALF OF CMW Geosciences (NZ) Limited Partnership Date 25/6/15
(Construction Review Firm)

Note: This statement shall only be relied upon by the Building Consent Authority named above. Liability under this statement accrues to the Design Firm only. The total maximum amount of damages payable arising from this statement and all other statements provided to the Building Consent Authority in relation to this building work, whether in contract, tort or otherwise (including negligence), is limited to the sum of \$200,000*.

This form is to accompany Forms 6 or 8 of the Building (Form) Regulations 2004 for the issue of a Code Compliance Certificate.

THIS FORM AND ITS CONDITIONS ARE COPYRIGHT TO ACENZ, ENGINEERING NEW ZEALAND AND NZIA